Appendix G: Future Conditions Traffic Analysis Memorandum



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#### **MEMORANDUM**

**Date**: April 10, 2017

To: Paul Vogel

**From**: Ross B. Tillman, P.E.

Kelsey E. Retherford, E.I.T.

**Subject**: Future Traffic Analysis

Riverfront Drive Corridor Study

Mankato/North Mankato Area Planning Organization

Project No.: T42.111867

#### Introduction

The Mankato/North Mankato Area Planning Organization in cooperation with the City of Mankato have requested a corridor study along Riverfront Drive from TH 14 to Woodland Avenue. Riverfront Drive is located along the western edge of the City of Mankato. This memorandum provides a summary of the future conditions and potential solutions.

#### **Traffic Forecasting**

Future traffic volumes for 2041 (25-yr forecast) were developed using historical data and the Mankato/North Mankato Area Planning Organization (MAPO) 2045 Long Range Transportation Plan while recognizing population growth trends in the area, which are likely to affect traffic volumes.

The historical growth rates (1997-2013) along Riverfront Drive were all calculated to be between 0.2 and 0.9 percent. An analysis was also completed using only the last 10 years of data but, the growth rates were all found to be negative. The MAPO 2045 Long Range Transportation Plan indicated future growth rates to be between 0.9 and 1.65 percent. Population growth trends have been between 0.5 percent per year and 1.5 percent per year on average depending on which time period is analyzed.

Traffic growth was compared using trend lines from various data sources and a 1% straight line growth value. These graphics are shown in **Figures 1** through **4** in **Appendix A**. In general, the 1% line falls within a similar range of the 2045 Long Range Transportation Plan and the full-history trend and would appear to provide a reasonable growth rate for the corridor. This assumption is meant to be all encompassing of background growth as well as spot redevelopment in areas nearby.

The historical growth rates for side streets with data available were also calculated. Data was available from 1997 to 2013 for the side streets, leading to historical growth rates ranging from -2.9 to 3 percent. The MAPO 2045 Long Range Transportation Plan had analysis at Cherry Street, Main Street and Madison Avenue east of Riverfront Drive. The growth rates were found to be between 1 and 1.9 percent in the Transportation Plan, however using the historical data growth rates at these intersections were between -2.9 and -0.5%. The historical side street data includes periods of time when streets were converted from one-way to two-way traffic and the reconstruction of 2<sup>nd</sup> Street and Mulberry Street

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occurred. These major circulation changes result in data patterns with no natural trends and essentially provide meaningless data.

Although the majority of analyzed side streets along the corridor are fully developed, they provide connectivity to other parts of the city that may experience development and growth. For this reason, a 1% per year growth rate was also applied to these approaches recognizing the potential for future growth in other areas and also the need to feed and receive a certain amount of traffic from Riverfront Drive.

Parallel routes to Riverfront Drive were also analyzed for spare capacity. TH 169 was projected to have 27,000 vehicles per day in 2045 as part of the Long Range Transportation Plan which is well under capacity for a 4-lane freeway. 2<sup>nd</sup> Street currently carries between 8,100 and 10,900 vehicles per day downtown, which if using the same growth rate as Riverfront Drive will also be under capacity in 2041.

#### **Future Operations Analysis**

A level of service (LOS) analysis of the peak hours was completed using the forecasted turning movement counts in SimTraffic. **Tables 1 through 4** show the results of the 2041 no build traffic analysis for Segments 1 through 4, respectively.

## Segment 1 – Woodland Avenue to Sibley Parkway

Table 1 - 2041 Existing Geometry (No Build) Traffic Operations Analysis

					<u>-</u>	Limiting	Max	Approach (	Queue
Intersection	Peak Hour		ection *- LOS	Maxi Delay-	-	Movement	Direction	Average Queue (ft)	Max Queue (ft)
Riverfront Dr & Woodland Ave	AM	2	Α	7	Α	NBT	NBL/T/R	25	100
Stop Controlled	PM	3	A	7	A	SBT	SBL/T/R	50	50
Riverfront Dr & Sibley St	AM	6	Α	41	Е	NBT	WBL/T	75	200
Stop Controlled	PM	7	Α	32	D	NBL	WBL/T	75	175
SB TH 169 Ramp/Owatonna St & Riverfront Dr	AM	49	D	75	Е	SBL	SBL/T	575	950
Signalized Intersection	PM	32	С	56	Е	SBT	SBL/T	275	650
NB TH 169 Ramp & Riverfront Dr	AM	62	F	1949	F	NBL	NBR	725	1275
Stop Controlled	PM	24	С	127	F	NBR	NBR	375	1000
Mankato West HS/Poplar St & Riverfront Dr	AM	79	Е	351	F	WBR	WBT/R	600	800
Signalized Intersection	PM	50	D	162	F	NBT	WBT/R	575	800
Riverfront Dr & Stoltzman Rd	AM	89	F	320	F	NBL	NBL	800	1300
Signalized Intersection	PM	44	D	156	F	NBT	NBL/T	425	500
Riverfront Dr & Marshall St	AM	75	Е	182	F	WBT	WBT	300	775
Signalized Intersection	PM	15	В	41	D	NBL	WBT	125	400

<sup>\*</sup>Delay in seconds per vehicle

#### AM Peak Hour

- Intersection delay has failing LOS at the intersections of Riverfront Drive with the NB TH 169 Ramp and Stoltzman Road.
- The limiting movement operates with LOS F at the following intersections:
  - o TH 169 North Ramp at Riverfront Drive
  - o Poplar Street-West Mankato High School at Riverfront Drive

<sup>\*\*</sup>Maximum delay and LOS on any approach and/or movement

<sup>\*\*\*</sup>Limiting Movement is the highest delay movement.

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Stoltzman Road at Riverfront Drive
 Marshall Street at Riverfront Drive

#### PM Peak Hour

- Intersection delay is acceptable with LOS D or better at all of the intersections.
- The limiting movement operates with LOS F at the following intersections:
  - o TH 169 North Ramp at Riverfront Drive
  - o Poplar Street-West Mankato High School at Riverfront Drive
  - o Stoltzman Road at Riverfront Drive

**Tables B1** and **B2** in **Appendix B** show the delay and queue lengths for each movement at all of the intersections in Segment 1.

## Segment 2 -Sibley Parkway to Veterans Memorial Bridge

Table 2 - 2041 Existing Geometry (No Build) Traffic Operations Analysis

						Limiting	Max	Approach (	Queue
Intersection	Peak Hour	1	ection *- LOS	Maxi Delay	mum ·LOS**	Movement	Direction	Average Queue (ft)	Max Queue (ft)
Riverfront Dr & Sibley Pkwy	AM	38	D	84	F	WBR	WBT	175	825
Signalized Intersection	PM	5	A	41	D	EBL	WBT	50	150
Riverfront Dr & Poplar St/Warren St	AM	14	В	32	С	WBL	NBT/R	100	275
Signalized Intersection	PM	18	В	31	C	EBL	SBT/R	150	350
Riverfront Dr & Minnesota St/Cherry St	AM	11	В	32	C	WBT	SBT	75	325
Signalized Intersection	PM	19	В	56	E	SBL	WBL/T	175	350
Riverfront Dr & Main St	AM	8	A	26	С	WBL	NBT	50	150
Signalized Intersection	PM	13	В	31	С	EBL	NBT	100	250

<sup>\*</sup>Delay in seconds per vehicle

#### AM Peak Hour

- Intersection delay is acceptable with LOS D or better at all of the intersections.
- The limiting movement operates with LOS F at the intersection of Sibley Parkway and Riverfront Drive.

#### PM Peak Hour

- Intersection delay is acceptable with LOS A or B at all of the intersections.
- The limiting movement operates with LOS E at the intersections of Minnesota Street-Cherry St and Riverfront Drive

**Tables B3** and **B4** in **Appendix B** show the delay and queue lengths for each movement at all of the intersections in Segment 2.

<sup>\*\*</sup>Maximum delay and LOS on any approach and/or movement

<sup>\*\*\*</sup>Limiting Movement is the highest delay movement.

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## Segment 3 -Veterans Memorial Bridge to Madison Avenue

Table 3 - 2041 Existing Geometry (No Build) Traffic Operations Analysis

			_			Limiting	Max	Approach (	Queue
Intersection	Peak Hour	Inters Delay	ection *- LOS	Maxi Delay	mum ·LOS**	Movement  ***	Direction	Average Queue (ft)	Max Queue (ft)
Riverfront Dr & Plum St	AM	4	A	23	C	WBL	NBT	50	175
Signalized Intersection	PM	5	Α	28	C	WBL	NBT	50	175
Riverfront Dr & Elm St	AM	4	A	11	В	WBL	NBL/T	50	125
Signalized Intersection	PM	6	A	22	C	EBL	SBL/T	75	225
Riverfront Dr & Madison Ave	AM	12	В	24	С	WBL	NBT/R	125	300
Signalized Intersection	PM	17	В	49	D	WBT	NBT/R	200	425

<sup>\*</sup>Delay in seconds per vehicle

#### AM Peak Hour

- Intersection delay is acceptable with LOS A or B at all of the intersections.
- The limiting movement is acceptable with LOS C or better at all of the intersections.

#### PM Peak Hour

- Intersection delay is acceptable with LOS A or B at all of the intersections.
- The limiting movement is acceptable with LOS D or better at all of the intersections.

**Tables B5** and **B6** in **Appendix B** show the delay and queue lengths for each movement at all of the intersections in Segment 3.

## Segment 4 – Madison Avenue to TH 14

Table 4 - 2041 Existing Geometry (No Build) Traffic Operations Analysis

						Limiting	Max	Approach C	Queue
Intersection	Peak Hour		ection *- LOS	Maxi Delay-		Movement	Direction	Average Queue (ft)	Max Queue (ft)
Riverfront Dr & 3rd Ave/Lafayette St	AM	5	A	57	F	EBL	EBR	75	225
Stop Controlled	PM	6	A	48	E	EBT	EBR	100	275
Riverfront Dr & May St	AM	1	A	16	C	WBL	WBL/R	25	75
Stop Controlled	PM	3	A	23	C	WBL	WBL/R	50	100
Riverfront Dr & TH 14 EB Ramp	AM	5	Α	22	C	EBL	EBR	100	250
Stop Controlled	PM	6	A	39	E	EBL	EBL	125	325
Riverfront Dr & TH 14 WB Ramp	AM	28	D	197	F	WBL	WBL/T	325	1025
Stop Controlled	PM	56	F	374	F	WBL	WBL/T	650	1725

<sup>\*</sup>Delay in seconds per vehicle

#### AM Peak Hour

• Intersection delay is acceptable with LOS D or better at all of the intersections.

<sup>\*\*</sup>Maximum delay and LOS on any approach and/or movement

<sup>\*\*\*</sup>Limiting Movement is the highest delay movement.

<sup>\*\*</sup>Maximum delay and LOS on any approach and/or movement

<sup>\*\*\*</sup>Limiting Movement is the highest delay movement.

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- The limiting movement operates with LOS F at the following intersections:
  - o 3<sup>rd</sup> Avenue/Lafayette Street at Riverfront Drive
  - o TH 14 Westbound Ramp at Riverfront Drive

#### PM Peak Hour

- Intersection delay and the limiting movement have failing LOS at the intersection of the TH 14 Westbound Ramp and Riverfront Drive.
- The intersection delay is LOS A for all other intersections.
- The limiting movement is LOS E at 3<sup>rd</sup> Avenue/Lafayette Street and the TH 14 Eastbound Ramp.
- The limiting movement is LOS C at May Street.

**Tables B7** and **B8** in **Appendix B** show the delay and queue lengths for each movement at all of the intersections in Segment 4.

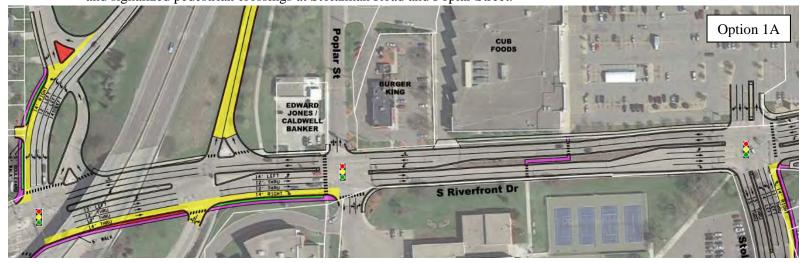
## **Alternative Concepts**

#### Segment 1 – Woodland Avenue to Sibley Parkway

There were six alternatives analyzed for Segment 1. These options are detailed below with snapshots of each.

## **Option 1: Traditional Signalized Corridor with Capacity and Pedestrian Enhancements**

Option 1A - Triple left turn from southbound TH 169, additional on ramp lane for northbound TH 169, Poplar Street remains signalized, turn lane additional and signal phasing improvement at Stoltzman Road, and signalized pedestrian crossings at Stoltzman Road and Poplar Street.



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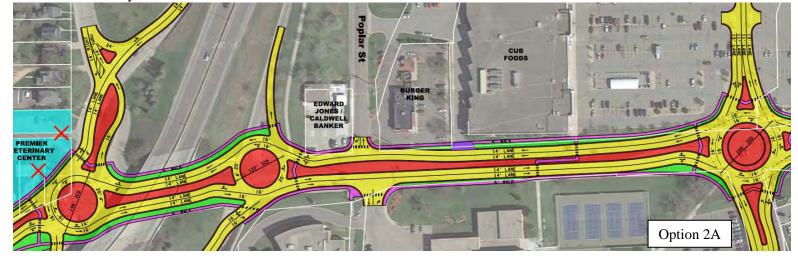
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Option 1B – No changes to the Southbound TH 169 Ramp, additional on ramp lane for northbound TH 169, partial signalization at Poplar Street with all left turns removed except the westbound left into Mankato West High School, turn lane additional and signal phasing improvement at Stoltzman Road, roadway expansion at Stoltzman Road through Cub Foods parking lot from Riverfront Drive to Sibley Parkway, and signalized pedestrian crossings at Stoltzman Road and Poplar Street.



Option 2: Roundabout Corridor with Access and Pedestrian Enhancements

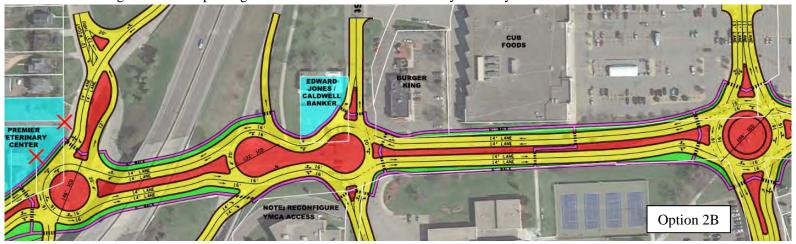
Option 2A - Roundabouts at TH 169 Ramps and Stoltzman Road, right-in/right-out at Poplar Street, and roadway expansion at Stoltzman Road through Cub Foods parking lot from Riverfront Drive to Sibley Parkway.



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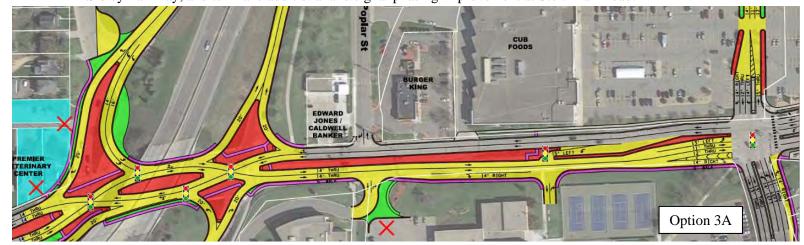
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Option 2B - Roundabouts at the Southbound TH 169 Ramp and Stoltzman Road, combined tear drop roundabout at the Northbound TH 169 Ramp and Poplar Street, and roadway expansion at Stoltzman Road through Cub Foods parking lot from Riverfront Drive to Sibley Parkway.



Option 3: Interchange Modifications with Signal at Stoltzman Road

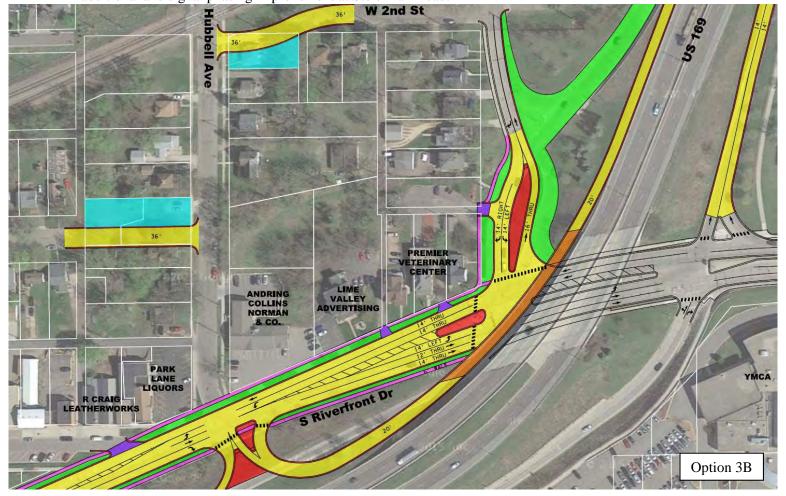
Option 3A - Diverging diamond at TH 169 Ramps, right-in/right-out at Poplar Street, additional on ramp lane for northbound TH 169, Mankato West High School entrance road shifted east and partially signalized, roadway extension at Stoltzman Road through Cub Foods parking lot from Riverfront Drive to Sibley Parkway, and turn lane additional and signal phasing improvement at Stoltzman Road.



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Option 3B - Loop ramp from southbound TH 169 eliminating access of Hubbell Avenue onto Riverfront Road, roadway extension of 2<sup>nd</sup> Street from Owatonna Street to Hubbell Avenue and 3<sup>rd</sup> Street between Sibley Street and Hubbell Avenue, additional on ramp lane for northbound TH 169, and turn lane additional and signal phasing improvement at Stoltzman Road.



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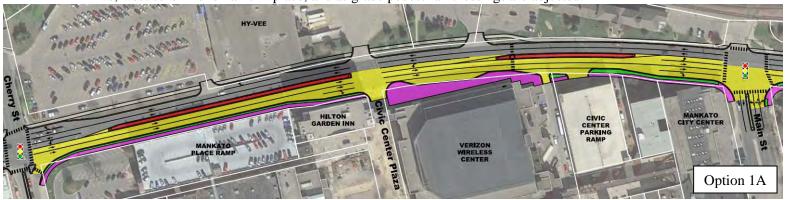
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## Segment 2 –Sibley Parkway to Veterans Memorial Bridge

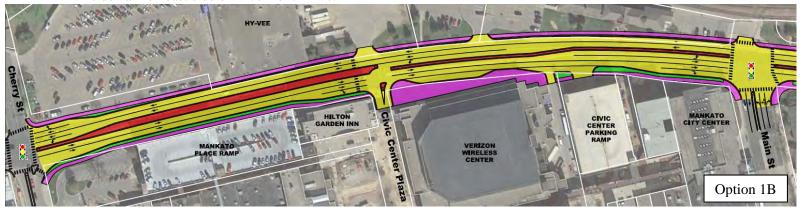
There were three alternatives analyzed for Segment 2. These options are detailed below with snapshots of each.

## Option 1: Four Lane Roadway with Spot Safety and Pedestrian Enhancements

Option 1A - The median is narrowed by eight feet to allow for a sidewalk on the east side of Riverfront Drive, west curb line remains in place, and at-grade pedestrian crossings are adjusted.

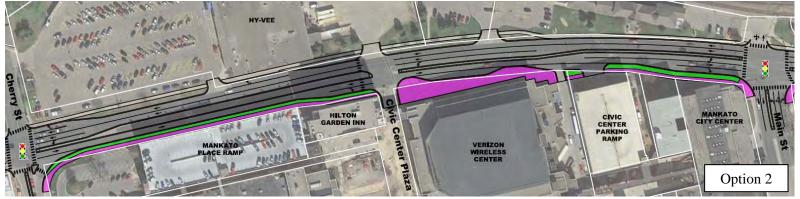


Option 1B - The west curb line between Cherry Street and Plum Street is shifted further west to provide a sidewalk on east side of Riverfront Drive.



**Option 2: Three Lane Roadway with Spot Safety and Pedestrian Enhancements** 

Northbound through lane is removed starting 50 feet north of Cherry Street to provide a sidewalk on the east side and at-grade pedestrian crossings are adjusted.



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## Segment 3 - Veterans Memorial Bridge to Madison Avenue

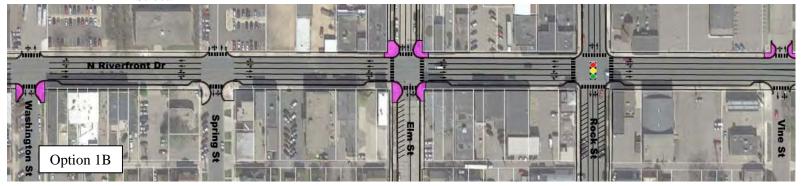
There were eight alternatives analyzed for Segment 3. These options are detailed below with snapshots of each.

#### Option 1: Four Lane Roadway with Spot Safety and Pedestrian Enhancements

Option 1A - Primary vehicle intersections at Plum Street & Elm Street and enhanced pedestrian corridor on Rock Street. Add bump-outs at Washington Street, Rock Street, and Vine Street.



Option 1B - Primary vehicle intersections at Plum Street & Rock Street and enhanced pedestrian corridor on Elm Street.



Option 2: Three Lane Roadway with Parking on South Side, Spot Safety and Pedestrian Enhancements

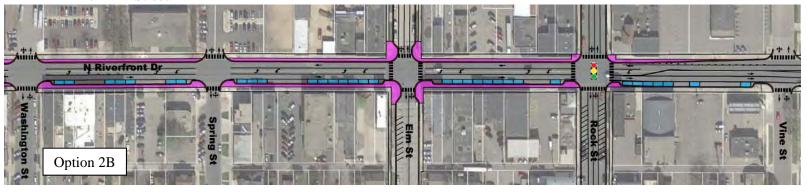
Option 2A - Primary vehicle intersections at Plum Street & Elm Street and enhanced pedestrian corridor on Rock Street.



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Option 2B - Primary vehicle intersections at Plum Street & Rock Street and enhanced pedestrian corridor on Elm Street.

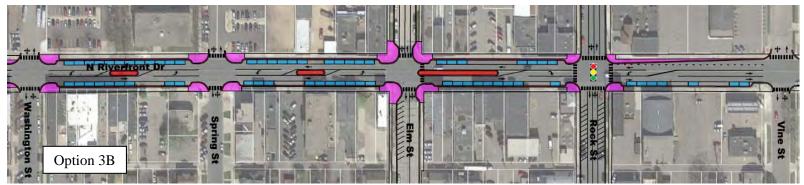


Option 3: Three Lane Roadway with Parking on Both Sides, Left Turn Lanes between Washington Street and Rock Street, Spot Safety and Pedestrian Enhancements

Option 3A - Primary vehicle intersections at Plum Street & Elm Street, enhanced pedestrian corridor on Rock Street and median refuge for pedestrians crossing Riverfront Drive.



Option 3B - Primary vehicle intersections at Plum Street & Rock Street, enhanced pedestrian corridor on Elm Street and median refuge for pedestrians crossing Riverfront Drive.



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## Option 4: Three Lane Roadway with Parking on South Side, Medians and Left Turn Lanes between Washington Street and Rock Street, Spot Safety and Pedestrian Enhancements

Option 4A - Primary vehicle intersections at Plum Street & Elm Street and enhanced pedestrian corridor on Rock Street.



Option 4B - Primary vehicle intersections at Plum Street & Rock Street and enhanced pedestrian corridor on Elm Street.



## Segment 4 - Madison Avenue to Good Council Drive

For analysis purposes TH 14 was analyzed separately as its own segment and Segment 4 was analyzed from Madison Avenue to Good Council Drive. There were three alternatives analyzed for Segment 4. These options are detailed below with snapshots of each.

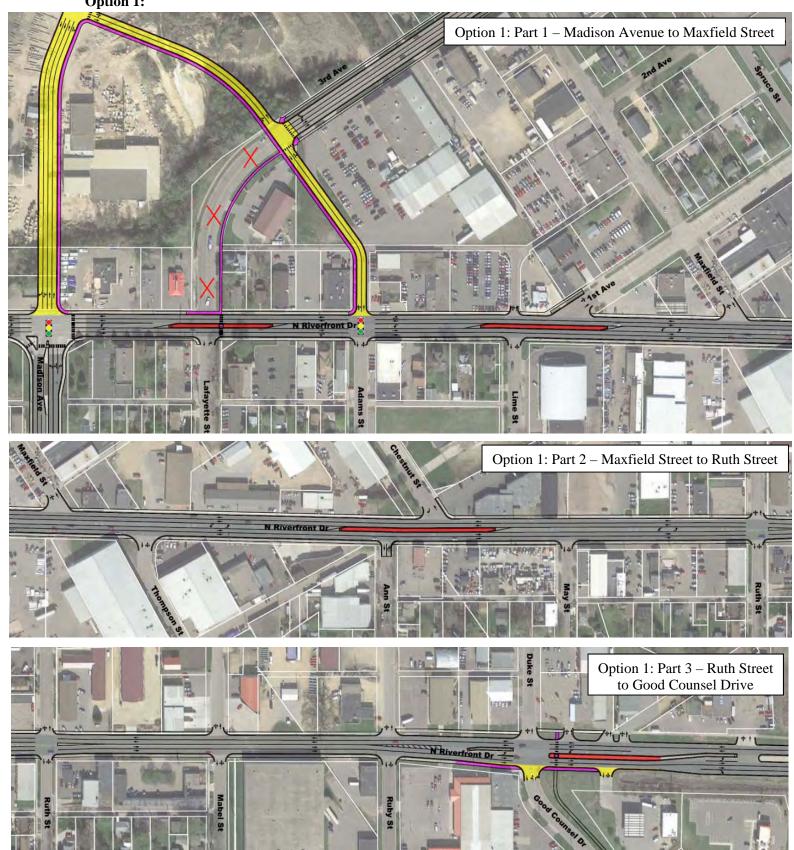
## Option 1: Primary Vehicle Intersections at Madison Avenue, Adams Street, May Street, and Good Council Drive

3<sup>rd</sup> Avenue is closed at Riverfront Drive, Madison Avenue is extended into current mining property and Adams Street is extended to the Madison Avenue extension. 3<sup>rd</sup> Avenue ties into Adams Street extension. Secondary intersections are converted to right-in/right-out. Pedestrian crossings at 3<sup>rd</sup> Avenue/Lafayette Street and Good Council Drive.

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Option 1:

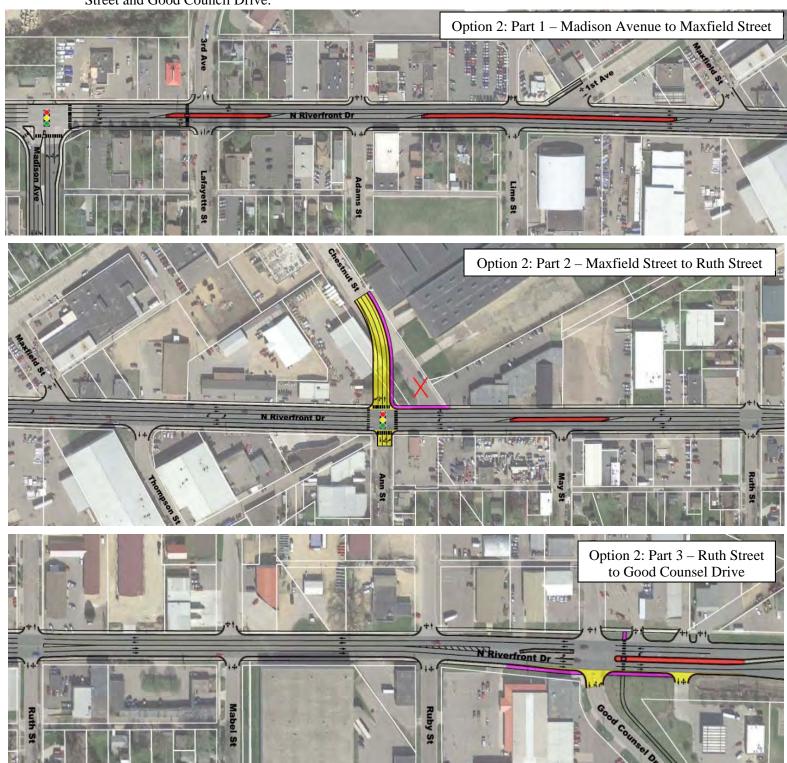


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# Option 2: Primary Vehicle Intersections at Madison Avenue, Chestnut Street/Ann Street, and Good Council Drive

3<sup>rd</sup> Avenue is converted to right-in/right-out. Chestnut Street is realigned and tied into Ann Street. Secondary intersections are converted to right-in/right-out. Pedestrian crossings at 3<sup>rd</sup> Avenue/Lafayette Street and Good Council Drive.

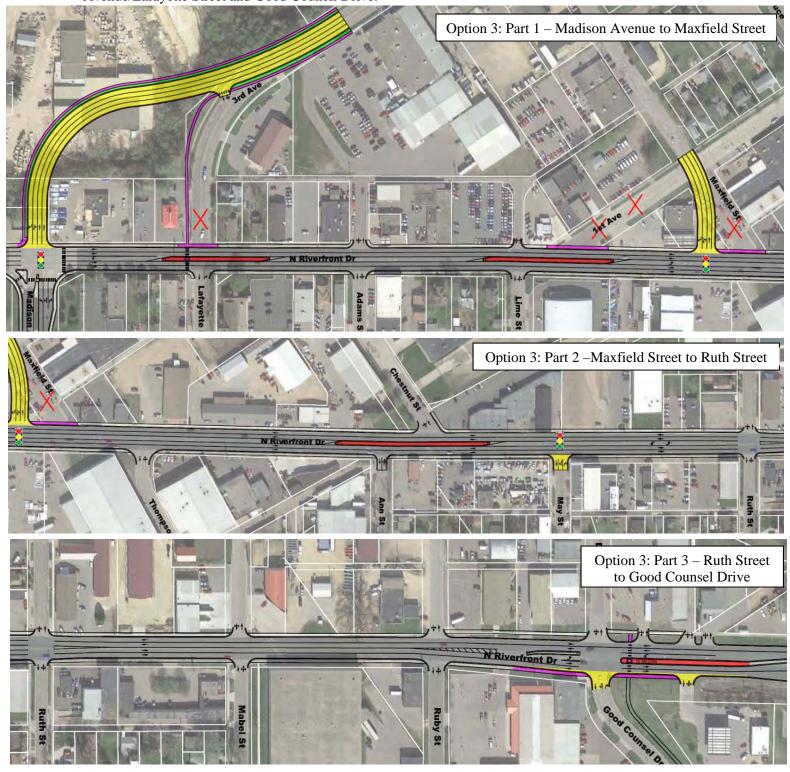


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# Option 3: Primary Vehicle Intersections at Madison Avenue, Maxfield Street, May Street, and Good Council Drive, realignment of 3rd Avenue to Madison Avenue Extension

3<sup>rd</sup> Avenue is realigned to tie into Madison Avenue, Maxfield Street is realigned to eliminate skewed intersection and May Street is upgraded as a primary intersection. Pedestrian crossings at 3<sup>rd</sup> Avenue/Lafayette Street and Good Council Drive.



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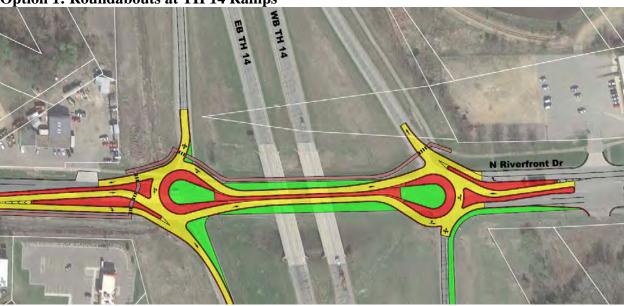
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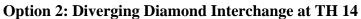
Additionally with each of the Segment 4 options there would be the option to keep the existing five lane section as shown in the snapshots, reduce the roadway to a three lane section with a center turn lane throughout the corridor or reduce the roadway to a two lane section with a median.

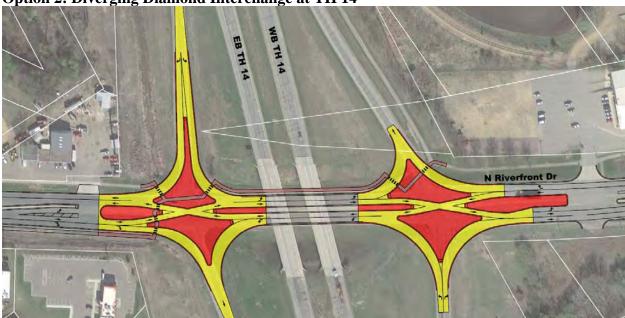
## Segment 5 – TH 14 Ramps

There were two alternatives analyzed for Segment 5. These options are detailed below with snapshots of each.









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## **Alternative Operations Analysis**

A traffic operational analysis was completed using the forecasted turning movement counts in SimTraffic for each option. **Tables 5** through **9** show the results of the 2041 no build traffic analysis compared to each option considered along each segment.

## Segment 1 -Woodland Avenue to Sibley Parkway

**Table 5. Segment 1 Traffic Operations Analysis** 

				Traditio	onal Sigr	nalized (	Corridor	Ro	undabo	ut Corri	dor	Inter	change I	Modifica	ations
Performance Measu	re	No E	Build	'	on 1A - e Lefts		on 1B - e Lefts	Th	n 2A - ree abouts	Fo	n 2B - ur labout	Optio Dive Diam	rging	Optio Loop	n 3B - Ramp
Vehicle Delay on	AM	44	D	11	В	13	В	9	Α	12	В	6	Α	10	Α
Riverfront (sec/veh)	PM	31	С	16	В	23	С	19	В	14	В	9	Α	19	В
Vehicle Delay on Side	AM	102	F	24	С	33	С	28	С	14	В	21	С	19	В
Streets (sec/veh)	PM	66	Е	34	С	41	D	57	Е	27	С	29	С	33	С
Segment Delay (veh-hr)	AM	23	30	5	57	7	71	5	55	5	0	6	0	5	1
Segment Delay (Ven-III)	PM	17	77	g	90	1.	25	1	27	7	9	9	6	10	08
Net Change in Eastbound	AM	4.2 (ba	seline)	-1	L.7	-1	L.5	-1	8	-1	7	-1	.4	-1	5
Travel Time (minutes)	PM	3.2 (ba	seline)	-0	).2	+(	0.3	-(	).4	-0	).7	-0	).2	+0	).4
Net Change in Westbound	AM	13 (ba	seline)	-1	0.7	-1	0.7	-1	0.8	-1	0.6	-10	0.3	-10	0.6
Travel Time (minutes)	PM	4.4 (ba	seline)	-1	L.6	-0	0.8	+(	).9	-1	5	-0	).8	-1	2
Intersection Capacity Util	ization	0.	92	0	).9	0.	94	1.	34	1.	37	0.	85	0.	92
Maximum Volume to Capac	ity Ratio	0.	95	0.	95	0.	91	1.	75	1.	52	0.	88	0.	99

**Table 5** shows that vehicle delay and segment delay is decreased overall for each option analyzed compared to the 2041 no build analysis. Travel time is increased by less than one minute during the PM peak hour for eastbound traffic in Options 1B and 3B and westbound in Option 2A and decreased for all other options. The intersection capacity utilization and maximum volume to capacity ratio is greater than one for the roundabout corridor options, but less than one for all other options.

**Tables C1** – C12 in **Appendix C** show the detailed delay and queue lengths for each option at all of the intersections in Segment 1.

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## Segment 2 –Sibley Parkway to Veterans Memorial Bridge

**Table 6. Segment 2 Traffic Operations Analysis** 

Table 6. Segment 2 Trail	ic Operai	nons A	11a1 y 515						
Performance Measu	ıre	No E	Build	Nar	ane	4 Lane	n 1B - Shifted Iway	Optio 3 La	on 2 - ane
Vehicle Delay on	AM	14	В	14	В	14	В	13	В
Riverfront (sec/veh)	PM	13	В	13	В	13	В	19	В
Vehicle Delay on Side	AM	14	В	14	В	14	В	28	С
Streets (sec/veh)	PM	18	В	18	В	18	В	32	С
Maximum Queue (ft)									
Poplar St/Warren St	AM	NBT	275	NBT	275	NBT	275	NBT	650
Popiai 3t/ Waiteli 3t	PM	NBT	275	NBT	275	NBT	275	NBT	625
Minnesota St/Cherry St	AM	NBT	225	NBT	225	NBT	225	NBT	375
Willinesota St/Cherry St	PM	NBT	300	NBT	300	NBT	300	NBT	375
Main St	AM	NBT	150	NBT	150	NBT	150	NBT	475
IVIdIII St	PM	NBT	250	NBT	250	NBT	250	NBT	775
Intersection Capacity Utiliz	ation	0.	79	0.	79	0.	79	0.	89
Maximum Volume to Capac	city Ratio	0.	83	0.	83	0.	83	0.	82

**Table 6** shows that operations are acceptable and the same as the existing no build analysis for Option 1A and Option 1B as no geometric changes were made at the intersections. With Option 2 vehicle delay increases, but remains acceptable at LOS C or better during both peak hours. Northbound queuing is problematic during the PM peak hours, however most vehicles do not wait more than one cycle. Delay is acceptable with LOS C or better at Minnesota Street/Cherry Street and Main Street and LOS D at Poplar Street/Warren Street. The intersection capacity utilization and maximum volume to capacity ratio is less than one for all options.

**Tables C13** – **C14** in **Appendix C** show the detailed delay and queue lengths at all of the intersections in Segment 2 for Option 2.

#### Segment 3 - Veterans Memorial Bridge to Madison Avenue

**Table 7. Segment 3 Traffic Operations Analysis** 

					Four	Lane		Three L	ane - Pa	rking Bot	h Sides			rking Bot Irn Lanes				king Sou Left Turr	,
Performance Meas	ure	No E	Build	Elm Pı	The Primary Intersection		on 1B rimary ection	Elm Pr	on 2A rimary ection	Option Rock P		Option Elm Pr Inters		Rock P	on 3B rimary ection	Elm Pr	on 4A rimary ection		on 4B rimary ection
Vehicle Delay on	AM	7	Α	7	7 A		Α	9	Α	8	Α	9	Α	8	Α	9	Α	8	Α
Riverfront (sec/veh)	PM	10	Α	10			Α	15	В	15	В	15	В	15	В	15	В	15	В
Vehicle Delay on Side	AM	14	В	14	В	14	В	20	С	20	В	20	С	20	В	20	С	20	В
Streets (sec/veh)	PM	18	В	18	В	18	В	30	C	30	С	30	C	30	С	30	C	30	С
Intersection Capacity Utiliz	ation	0.	74	0.	74	0.	74	0.	81	0.	82	0.	81	0.	82	0.	81	0.8	82
Maximum Volume to Capa	city Ratio	0.	78	0.	78	0.	78	1.	05	1.	05	1.	05	1.	05	1.	05	1.0	05

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**Table 7** shows that vehicle delay is acceptable for all options with LOS C or better. The intersection capacity utilization is less than one for all options. The maximum volume to capacity ratio is greater than one for the three lane options and less than one for the four lane option.

**Tables C15** and **C16** in **Appendix C** show the detailed delay and queue lengths at all of the intersections in the three lane options with Elm Street as a primary intersection. **Tables C17** and **C18** in **Appendix C** show the detailed delay and queue lengths at all of the intersections in the three lane options with Rock Street as a primary intersection.

Madison Avenue at Riverfront Drive was also analyzed as a roundabout to determine if the three lane section could begin north of Madison Avenue however the volumes were too high in this area for a single lane roundabout to function with acceptable delay.

Additionally, analysis was completed to determine if the free northbound right turn could be eliminated at Plum Street. The movement delay was found to be 4.5 seconds with the free right and 4.7 seconds without the free right so it is recommended that the free right be removed to improve pedestrian safety.

#### Segment 4 – Madison Avenue to Good Council Drive

Traffic operations were not analyzed for the alternatives discussed along Segment 4. Due to the lower traffic volumes it was assumed that operations would be comparable between all of the options. Instead these options were analyzed with the goals of how to provide efficient vehicle and freight mobility and access, safety for all users, infrastructure improvements compatible with the historic and natural environment and how to enhance the community identity. Details on how each option is able to meet these goals is described in the Final Study Report.

#### Segment 5 – TH 14 Ramps

**Table 9. Segment 5 Traffic Operations Analysis** 

Performance Meas	ure	No E	Build	Optio Round	on 1 - abouts	Option Diverging	on 2 - Diamond
Vehicle Delay on	AM	2	А	23	С	6	А
Riverfront (sec/veh)	PM	2	Α	16	С	9	Α
Vehicle Delay on Ramps	AM	99	F	20	С	5	Α
(sec/veh)	PM	193	F	20	С	5	Α
Intersection Capacity Utiliz	ation	0.	54	1.	12	0.	73
Maximum Volume to Capac	city Ratio	3.	01	0.	99	0.	78

**Table 9** shows that with roundabouts at both ramps in Option 1 the vehicle delay on Riverfront Drive increases, but remains acceptable at LOS C. With the diverging diamond in Option 2 the vehicle delay on Riverfront Drive remains LOS A. The vehicle delay on the ramps is significantly less for both options compared to 2041 no build analysis. The intersection capacity utilization is greater than one for Option 1 and remains less than one for Option 2. The maximum volume to capacity ratio is greatly reduced and less than one for both options.

Appendix A: Traffic Forecasting

Figure 1. Traffic Growth at Riverfront Drive West of Stoltzman Road

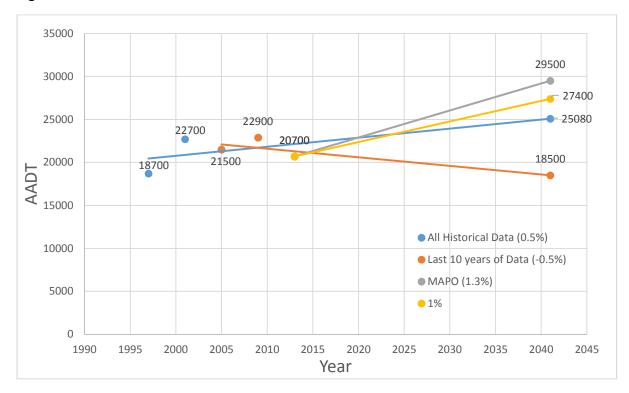


Figure 2. Traffic Growth at Riverfront Drive South of Minnesota Street-Cherry Street

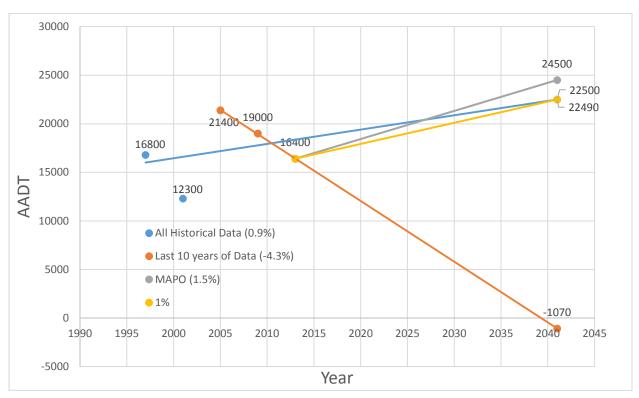


Figure 3. Traffic Growth at Riverfront Drive South of Lafayette Street-3<sup>rd</sup> Avenue

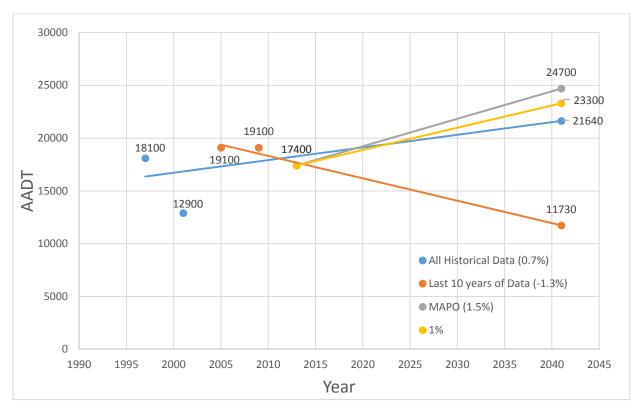
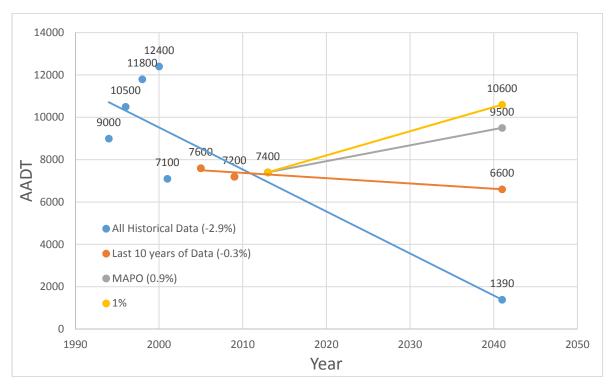


Figure 4. Traffic Growth at Riverfront Drive North of TH 14 Westbound Ramp



Appendix B: 2041 No Build Traffic Operational Analysis

Table B1: 2041 Segment 1 Existing Geometry Traffic Operations Analysis - Riverfront Drive Corridor Study

Intersection	Peak	Inters	ection											Mover	nent D	elay (s	ec/vel	1)									
intersection	Hour	Delay	*- LOS	E	BL	EI	ВТ	EE	3R	W	BL	V	ВТ	W	BR	N	BL	N	ВТ	N	BR	S	BL	SI	3T	SI	BR
Riverfront Dr & Woodland Ave	AM	2	Α		-	0	A	0	A	2	A	1	A	1	A	6	A	7	A	2	Α	5	A	7	A		-
Stop Controlled	PM	3	Α	2	Α	0	Α	0	A	3	Α	1	Α	1	Α	6	Α	7	Α	2	Α	5	Α	7	Α	3	Α
Riverfront Dr & Sibley St	AM	6	A		-	4	A	4	Α	9	A	2	A	2	A	33	D	41	Е	4	A	16	C	15	C	7	A
Stop Controlled	PM	7	Α	7	A	4	Α	4	A	9	A	2	A	2	A	32	D	27	D	2	Α	25	D	19	C	8	Α
SB TH 169 Ramp/Owatonna St & Riverfront Dr	AM	49	D	14	В	34	C		-	22	C	17	В	2	A		-		-		-	75	Е		-	41	D
Signalized Intersection	PM	32	C	8	A	43	D	33	С	26	C	16	В	3	A		-		-		-	43	D	56	Е	16	В
NB TH 169 Ramp & Riverfront Dr	AM	62	F	7	A	14	В		-		-	14	В	37	Е	1949	F	514	F	748	F		-		-		-
Stop Controlled	PM	24	C	24	С	6	Α		-		-	5	Α	17	C		-	77	F	127	F		-		-		-
Mankato West HS/Poplar St & Riverfront Dr	AM	79	E	11	В	19	В	22	C	59	Е	156	F	351	F	261	F	253	F	153	F	78	Е	86	F	111	F
Signalized Intersection	PM	50	D	22	C	18	В	19	В	62	Е	69	Е	103	F	134	F	162	F	65	Е	73	Е	60	Е	60	E
Riverfront Dr & Stoltzman Rd	AM	89	F	16	В	16	В	12	В	19	В	83	F	16	В	320	F	268	F	69	Е	16	В	26	С	44	D
Signalized Intersection	PM	44	D	46	D	32	C	15	В	42	D	39	D	10	В	117	F	156	F	19	В	26	C	37	D	24	C
Riverfront Dr & Marshall St	AM	75	E		-	7	A	4	A	128	F	182	F		-	176	F		-	1	Α		-		-		-
Signalized Intersection	PM	15	В		-	7	A	4	A	23	C	18	В		-	41	D		-	1	A		-		-		-

<sup>\*</sup>Delay in seconds per vehicle

Table B2: 2041 Segment 1 Existing Geometry Peak Hour Queues By Movement

	Daal											Q	ueue L	ength:	s										
Intersection	Peak Hour	E	BL	E	ВТ	Е	BR	W	BL	W	ВТ	W	BR	N	BL	N	ВТ	N	BR	S	BL	SI	ВТ	SI	BR
	noui	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max										
Riverfront Dr & Woodland Ave	AM	0	25	0	25	0	25	25	50	25	50	-	-	25	100	25	100	25	100	25	50	25	50	25	50
Stop Controlled	PM	0	25	0	25	0	25	25	50	25	50	1	-	25	25	25	25	25	25	50	50	50	50	50	50
Riverfront Dr & Sibley St	AM	25	75	25	75	25	75	75	200	75	200	25	75	50	125	50	125	-	-	25	50	25	50	25	50
Stop Controlled	PM	25	50	25	50	25	50	75	175	75	175	25	25	25	75	25	75	-	-	25	75	25	75	25	75
SB TH 169 Ramp/Owatonna St & Riverfront Dr	AM	25	25	200	350	200	350	75	150	25	125	25	125	i	-	-	-	-	-	575	950	1	-	175	300
Signalized Intersection	PM	0	25	150	300	150	300	200	275	75	150	75	150	1	-	-	i	-	-	275	650	-	-	125	300
NB TH 169 Ramp & Riverfront Dr	AM	75	250	200	300	-	-	-	-	200	250	175	200	25	150	25	150	725	1275	-	-	-	-	·	-
Stop Controlled	PM	100	250	100	275	-	-	-	-	250	275	200	225	25	100	25	100	375	1000	-	-	-	-	-	-
Mankato West HS/Poplar St & Riverfront Dr	AM	50	150	200	225	200	225	75	200	600	800	600	800	350	525	350	525	100	150	150	375	150	375	150	375
Signalized Intersection	PM	75	225	225	250	225	250	125	250	575	800	575	800	225	500	225	500	100	150	200	450	200	450	200	450
Riverfront Dr & Stoltzman Rd	AM	25	75	100	200	100	200	50	150	175	275	25	175	800	1300	-	-	725	1300	25	75	i	-	25	100
Signalized Intersection	PM	125	225	150	300	125	225	150	250	200	300	100	200	425	500	-	i	325	475	75	125	-	-	100	125
Riverfront Dr & Marshall St	AM	-	-	75	175	50	125	50	175	300	775	-	-	150	525	-	-	-	-	-	-	-	-	-	-
Signalized Intersection	PM	-	-	75	250	50	150	50	175	125	400	-	-	125	250	-	-	0	25	-	-	-	-	-	-

<sup>\*\*</sup>Maximum delay and LOS on any approach and/or movement

<sup>\*\*\*</sup>Limiting Movement is the highest delay movement.

Table B3: 2041 Segment 2 Existing Geometry Traffic Operations Analysis - Riverfront Drive Corridor Study

Intersection	Peak	Inters	ection											Moven	nent D	elay (s	ec/vel	1)									
intersection	Hour	Delay	*- LOS	El	BL	El	3T	EE	3R	W	BL	W	ВТ	W	BR	NI	BL	N	ВТ	N	BR	S	BL	SI	ВТ	SI	BR
Riverfront Dr & Sibley Pkwy	AM	38	D	39	D	3	A		-		-	74	Е	84	F		-		-		-	33	C		-	48	D
Signalized Intersection	PM	5	Α	41	D	4	Α		-		-	5	A	4	Α		-		-		-	26	С		-	7	Α
Riverfront Dr & Poplar St/Warren St	AM	14	В	30	C	26	C	21	C	32	C	24	C	10	В	10	В	10	В	9	A	19	В	12	В	11	В
Signalized Intersection	PM	18	В	31	C	30	C	22	C	29	C	25	C	11	В	28	C	15	В	13	В	24	C	15	В	12	В
Riverfront Dr & Minnesota St/Cherry St	AM	11	В	28	C	30	C	13	В	30	C	32	C	2	A	10	В	6	Α	4	A	22	C	10	В	6	Α
Signalized Intersection	PM	19	В	25	C	20	С	11	В	30	C	29	C	3	Α	27	С	13	В	10	В	56	Е	18	В	9	Α
Riverfront Dr & Main St	AM	8	A	26	C		-		-	26	C		-	7	A	6	A	8	Α	5	A	15	В	5	A	5	Α
Signalized Intersection	PM	13	В	31	C		-	8	A	26	C	18	В	9	Α	17	В	13	В	9	Α	23	C	9	Α		-

<sup>\*</sup>Delay in seconds per vehicle

Table B4: 2041 Segment 2 Existing Geometry Peak Hour Queues By Movement

	Daal											Q	ueue L	ength:	s										
Intersection	Peak Hour	El	BL	E	ВТ	EI	BR	W	BL	W	ВТ	W	BR	N	BL	N	ВТ	N	BR	S	BL	S	ВТ	SE	BR
	Hour	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max										
Riverfront Dr & Sibley Pkwy	AM	25	50	25	125	-	-	-	-	175	825	25	150	-	-	-	-	-	-	25	75	-	-	25	50
Signalized Intersection	PM	25	75	50	125	-	-	-	1	50	150	25	50	ı	-	1	-	•	-	50	100	-	-	25	50
Riverfront Dr & Poplar St/Warren St	AM	75	200	75	200	75	200	50	125	50	150	50	150	25	100	100	275	100	275	50	150	75	275	75	275
Signalized Intersection	PM	125	250	125	250	125	250	75	175	75	150	75	150	50	150	125	275	125	275	75	225	150	350	150	350
Riverfront Dr & Minnesota St/Cherry St	AM	50	100	25	100	25	100	100	225	100	225	25	75	25	50	75	200	75	200	50	100	75	325	25	75
Signalized Intersection	PM	75	125	50	150	50	150	175	350	175	350	50	125	50	150	125	300	125	300	75	225	150	325	50	175
Riverfront Dr & Main St	AM	25	25	25	25	25	25	75	150	75	150	50	125	25	50	50	150	25	50	25	100	50	125	50	125
Signalized Intersection	PM	25	75	25	75	25	75	100	175	100	175	50	100	25	50	100	250	50	175	50	125	75	200	75	200

<sup>\*\*</sup>Maximum delay and LOS on any approach and/or movement

<sup>\*\*\*</sup>Limiting Movement is the highest delay movement.

Table B5: 2041 Segment 3 Existing Geometry Traffic Operations Analysis - Riverfront Drive Corridor Study

Intersection	Peak	Inters	ection									ı	Moven	nent D	elay (s	ec/veł	1)									
intersection	Hour	Delay	*- LOS	E	BL	EI	3T	E	3R	W	BL	WBT	W	BR	NI	BL	N	ВТ	N	BR	SI	BL	SI	ВТ	SE	BR
Riverfront Dr & Plum St	AM	4	A		-		EBT -		-	23	C	1	7	A	15	В	5	A	3	A	8	A	3	Α	3	A
Signalized Intersection	PM	5	Α	10	В		-		-	28	C	ı	9	Α	-	-	5	Α	5	Α	13	В	3	Α		- '
Riverfront Dr & Elm St	AM	4	A	10	В	6	Α	4	A	11	В	-	6	Α	10	В	4	A	2	A	6	Α	3	Α	3	A
Signalized Intersection	PM	6	A	22	C	21	С	9	Α	18	В	ı	7	Α	14	В	6	Α	5	A	16	В	6	Α	4	Α
Riverfront Dr & Madison Ave	AM	12	В		-		-		-	24	C	ı	10	В		-	14	В	8	A	22	C	6	Α		-
Signalized Intersection	PM	17	В		-		-		-	26	C	49 D	12	В		-	25	C	14	В	22	C	8	Α	9	A

<sup>\*</sup>Delay in seconds per vehicle

Table B6: 2041 Segment 3 Existing Geometry Peak Hour Queues By Movement

	Doole											C	Queue L	.ength	s										
Intersection	Peak Hour	E	BL	Е	ВТ	E	BR	W	/BL	W	ВТ	W	/BR	N	BL	N	ВТ	N	BR	S	BL	SI	ВТ	SF	BR
	Hour	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max										
Riverfront Dr & Plum St	AM	-	-	-	-	-	-	25	50	25	50	50	75	25	25	50	175	25	25	25	75	25	75	25	75
Signalized Intersection	PM	25	25	25	25	25	25	25	75	25	75	50	75	-	-	50	175	25	50	50	125	25	100	25	100
Riverfront Dr & Elm St	AM	25	50	25	50	25	50	25	50	25	50	-	-	50	125	50	125	50	125	25	125	25	125	_	-
Signalized Intersection	PM	50	125	50	125	50	125	25	50	25	50	-	-	75	175	75	175	75	175	75	225	75	225	-	-
Riverfront Dr & Madison Ave	AM	-	-	-	-	-	-	75	125	25	150	75	150	-	-	125	300	125	300	75	200	75	225	75	225
Signalized Intersection	PM	-	-	-	-	-	-	125	200	100	200	100	175	-	-	200	425	200	425	100	200	75	175	75	175

<sup>\*\*</sup>Maximum delay and LOS on any approach and/or movement

<sup>\*\*\*</sup>Limiting Movement is the highest delay movement.

Table B7: 2041 Segment 4 Existing Geometry Traffic Operations Analysis - Riverfront Drive Corridor Study

Intersection	Peak	Inters	ection										ı	Moven	nent D	elay (s	ec/vel	1)									
intersection	Hour	Delay	*- LOS	El	BL	E	ВТ	EE	3R	W	BL	WE	ВТ	W	BR	N	BL	N	ВТ	N	BR	S	BL	SI	3T	SE	ЗR
Riverfront Dr & 3rd Ave/Lafayette St	AM	5	A	57	F	25	D	15	C		-	45	Е	12	В	10	В	2	Α	2	Α	8	A	3	Α	2	A
Stop Controlled	PM	6	A	48	Е	48	Е	17	C		-	-		6	A	9	A	3	Α	3	Α	8	A	3	A	1	A
Riverfront Dr & May St	AM	1	A		-		-		-	16	C	-		6	Α		-	2	A	2	A	3	A	0	Α	-	
Stop Controlled	PM	3	A		-		-		-	23	C	-	ì	11	В		-	3	Α	3	Α	6	A	0	A		-
Riverfront Dr & TH 14 EB Ramp	AM	5	A	22	C		-	13	В		-	-			-		-	1	Α	2	Α	5	A	1	Α		
Stop Controlled	PM	6	A	39	Е		-	7	A		-	-	ì		-		-	2	Α	3	Α	12	В	1	Α		-
Riverfront Dr & TH 14 WB Ramp	AM	28	D		-		-		-	197	F	-		80	F	11	В	1	A		-		-	1	Α	1	A
Stop Controlled	PM	56	F		-		-		-	374	F	-		242	F	6	A	1	A		-		-	1	Α	1	Α

<sup>\*</sup>Delay in seconds per vehicle

Table B8: 2041 Segment 4 Existing Geometry Peak Hour Queues By Movement

	Deal											Q	ueue L	ength.	s										
Intersection	Peak Hour	E	BL	E	ВТ	EI	BR	W	BL.	W	ВТ	W	BR	N	BL	N	ВТ	N	BR	S	BL	S	ВТ	SI	BR
	noui	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max
Riverfront Dr & 3rd Ave/Lafayette St	AM	25	50	-	-	75	225	50	75	50	75	-	-	75	150	25	25	25	25	25	75	25	25	25	25
Stop Controlled	PM	25	50	1	-	100	275	25	50	25	50	-	-	75	150	-	-	-	-	25	50	-	-	-	-
Riverfront Dr & May St	AM	-	-	-	-	-	-	25	75	-	-	25	75	-	-	-	-	-	-	25	50	-	-	-	-
Stop Controlled	PM	-	1	1	-	-	-	50	100	-	ı	50	100	1	-	0	25	0	25	25	50	-	-	-	-
Riverfront Dr & TH 14 EB Ramp	AM	75	225	-	-	100	250	-	-	-	-	-	-	-	-	-	-	0	25	25	75	-	-	-	-
Stop Controlled	PM	125	325	-	-	75	200	-	-	-	-	-	-	-	-	-	-	25	25	25	75	-	-	-	-
Riverfront Dr & TH 14 WB Ramp	AM	-	-	-	-	-	-	325	1025	325	1025	100	350	50	175	-	-	-	-	-	-	-	-	0	25
Stop Controlled	PM	-	-	-	-	-	-	650	1725	650	1725	150	350	50	175	-	-	-	-	-	-	-	-	25	25

<sup>\*\*</sup>Maximum delay and LOS on any approach and/or movement

<sup>\*\*\*</sup>Limiting Movement is the highest delay movement.

Appendix C: 2041 Alternative Traffic Operational Analysis

Table C1: 2041 Segment 1 - Option 1A Traffic Operations Analysis

														Mover	nent D	elay (se	c/veh)										
Intersection	Peak Hour	Inters Delay	ection * - LOS	EI	BL	E	ВТ	El	BR	w	BL	w	ВТ	w	BR	N	BL	N	ВТ	N	BR	SI	BL	SI	зт	SI	BR
Riverfront Dr & Woodland Ave	AM	2	A		-	0	A	0	A	2	A	1	A	1	A	6	A	8	A	2	A	6	A	6	A		-
Stop Controlled	PM	3	A	3	A	1	A	0	A	3	A	1	A	1	A	6	A	8	A	2	A	6	A	7	A	2	A
Riverfront Dr & Sibley St	AM	9	A		-	5	A	4	A	11	В	2	A	1	A	68	F	66	F	7	A	24	C	17	C	11	В
Stop Controlled	PM	8	A	5	A	4	A	4	A	11	В	2	A	2	Α	38	Е	29	D	2	A	32	D	26	D	13	В
Riverfront Dr & SB TH 169 Ramps/Owatonna St	AM	27	C	13	В	31	C		-	24	C	12	В	2	A		-		-		-	33	C			13	В
Signalized Intersection	PM	32	C	34	С	46	D	41	D	29	С	15	В	3	A		-		-		-	42	D	43	D	12	В
Riverfront Dr & NB TH 169 Ramps	AM	6	A	9	A	3	A		-			2	A	3	A	84	F	23	C	36	E						-
Stop Controlled	PM	12	В	24	C	4	A		-		-	4	A	4	A		-	48	E	64	F						-
Mankato West HS/Poplar St & Riverfront Dr	AM	22	C	18	В	14	В	6	Α	54	D	16	В	16	В	62	E	59	E	41	D	32	C	28	C	21	C
Signalized Intersection	PM	25	С	29	C	14	В	3	Α	58	E	23	C	21	C	62	E	63	E	28	C	54	D	46	D	42	D
Riverfront Dr & Stoltzman Rd	AM	16	В	16	В	17	В	17	В	16	В	12	В	3	Α	22	C	19	В	9	A	18	В	27	C	8	A
Signalized Intersection	PM	28	С	42	D	29	C	16	В	36	D	27	C	7	Α	36	D	38	D	11	В	34	C	49	D	20	C
Riverfront Dr & Marshall St	AM	5	A		-	5	A	3	A	14	В	4	A		-	13	В		-	1	A				-		-
Signalized Intersection	PM	11	В		-	6	A	3	A	22	C	9	A		-	41	D		-	1	A		-		-		-

<sup>\*</sup>Delay in seconds per vehicle

Table C2: 2041 Segment 1 - Option 1A Peak Hour Queues By Movement

	Dools											Qu	eue Le	ngths (1	ft)										
Intersection	Peak Hour	Е	BL	EE	3T	EI	BR	W	'BL	W	ВТ	W	BR	N	BL	N	ВТ	N	BR	S	BL	SI	ВТ	SI	BR
	noui	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max										
Riverfront Dr & Woodland Ave	AM	0	25	0	25	0	25	25	50	25	50	-	-	25	50	25	50	25	50	25	50	25	50	25	50
Stop Controlled	PM	0	25	0	25	0	25	25	50	25	50	-	-	25	25	25	25	25	25	50	75	50	75	50	75
Riverfront Dr & Sibley St	AM	25	50	25	50	25	50	75	200	75	200	75	200	75	200	75	200	25	50	25	50	25	50	25	50
Stop Controlled	PM	25	50	25	50	25	50	100	300	100	300	100	300	25	75	25	75	-	-	25	100	25	100	25	100
Riverfront Dr & SB TH 169 Ramps/Owatonna St	AM	25	75	200	375	200	375	100	200	25	100	25	100	-	-	-	-	-	-	150	500	150	500	75	275
Signalized Intersection	PM	25	25	175	275	175	275	200	300	75	150	75	150	-	-	-	-	-	-	175	550	175	550	100	275
Riverfront Dr & NB TH 169 Ramps	AM	75	225	50	175	-	-	-	-	25	150	25	125	25	25	25	25	100	275	-	-	-	-	-	-
Stop Controlled	PM	100	225	25	200	-	-	-	-	50	250	25	200	25	25	25	25	225	700	-	-	-	-	-	-
Mankato West HS/Poplar St & Riverfront Dr	AM	50	150	175	250	100	200	125	275	175	400	175	400	225	625	225	625	100	150	75	175	75	175	75	175
Signalized Intersection	PM	75	175	175	250	50	100	125	250	275	500	275	500	150	475	150	475	75	150	175	350	175	350	175	350
Riverfront Dr & Stoltzman Rd	AM	50	125	125	450	150	225	50	125	75	175	25	100	125	250	50	225	75	175	-	-	-	-	25	75
Signalized Intersection	PM	125	225	125	300	125	225	150	250	175	275	75	200	175	275	100	375	75	150	-	-	-	-	75	125
Riverfront Dr & Marshall St	AM	-	-	50	125	50	100	25	75	50	125	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Signalized Intersection	PM	-	-	75	200	50	150	50	150	100	275	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Table C3: 2041 Segment 1 - Option 1B Traffic Operations Analysis

														Mover	nent D	elay (se	ec/veh	)										
Intersection	Peak Hour		ection * - LOS	EBU	E	BL	E	вт	Е	BR	w	BL	w	вт	w	BR	N	BL	NI	вт	N	BR	S	BL	SI	вт	SI	BR
Riverfront Dr & Woodland Ave	AM	2	A	-		-	0	A	0	Α	3	Α	1	A	1	A	6	Α	8	A	2	Α	5	A	7	A		-
Stop Controlled	PM	3	A	-	2	A	0	Α	0	Α	3	Α	1	A	1	Α	7	Α	8	A	2	Α	6	A	7	Α	4	Α
Riverfront Dr & Sibley St	AM	7	A	-		-	4	A	4	Α	9	Α	2	A	1	A	43	Е	46	Е	5	Α	19	C	17	C	11	В
Stop Controlled	PM	7	A	-	8	A	4	Α	4	A	10	В	2	A	1	A	29	D	24	C	2	Α	25	D	23	С	11	В
Riverfront Dr & SB TH 169 Ramps/Owatonna St	AM	42	D		16	В	33	C	22	C	26	C	7	A	2	A		-				-	64	E			34	C
Signalized Intersection	PM	34	C	-	19	В	44	D	16	В	31	C	13	В	4	Α		-				-	46	D			16	В
Riverfront Dr & NB TH 169 Ramps	AM	11	В	-	9	A	13	В		-			7	A	3	A	23	C	66	Е	28	C		-				-
Signalized Intersection	PM	14	В	-	16	В	11	В		-		-	14	В	4	A	44	D	42	D	35	D		-				-
Mankato West HS/Poplar St & Riverfront Dr	AM	16	В	-		-	16	В	9	Α	23	C	6	A	5	Α		-			39	D					55	E
Signalized Intersection	PM	22	C	-		-	14	В	5	Α	38	D	15	В	18	В		-			65	E		-			51	D
Riverfront Dr & Stoltzman Rd	AM	21	C	28 C	30	C	17	В	14	В	20	C	25	C	5	Α	33	C	22	C	9	Α	41	D	42	D	14	В
Signalized Intersection	PM	42	D	97 F	80	F	33	C	14	В	38	D	55	Е	18	В	70	E	38	D	9	Α	54	D	50	D	24	C
Riverfront Dr & Marshall St	AM	6	A	-		-	3	A	2	Α	11	В	4	A		-	37	D			1	Α		-				-
Signalized Intersection	PM	21	C	-		-	14	В	6	A	37	D	48	D		-	85	F			1	A		-				-

<sup>\*</sup>Delay in seconds per vehicle

Table C4: 2041 Segment 1 - Option 1B Peak Hour Queues By Movement

	Peak											Qu	eue Le	ngths (	ft)										
Intersection	Реак Hour	Е	BL	Е	ВТ	E	BR	W	/BL	W	ВТ	W	BR	N	BL	N	ВТ	N	BR	S	BL	SI	ВТ	SI	BR
	Houi	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max										
Riverfront Dr & Woodland Ave	AM	-	-	-	-	-	-	25	50	25	50	25	50	25	75	25	75	25	75	25	50	25	50	25	50
Stop Controlled	PM	25	25	25	25	25	25	25	75	25	75	25	75	25	50	25	50	25	50	50	75	50	75	50	75
Riverfront Dr & Sibley St	AM	25	75	25	75	25	75	75	175	75	175	75	175	50	200	50	200	25	50	25	50	25	50	25	50
Stop Controlled	PM	25	50	25	50	25	50	75	250	75	250	75	250	25	75	25	75	-	-	25	100	25	100	25	100
Riverfront Dr & SB TH 169 Ramps/Owatonna St	AM	25	25	175	325	175	325	75	150	25	75	25	75	-	-			-	-	550	950	550	950	175	300
Signalized Intersection	PM	0	25	150	250	150	250	225	275	50	150	50	150	-	-	1	ı	-	-	300	775	300	775	125	300
Riverfront Dr & NB TH 169 Ramps	AM	100	225	175	300	-	-	-	-	50	175	25	175	25	50	25	50	100	225	-	-	-	-	-	-
Signalized Intersection	PM	100	200	150	275	-	-	-	-	125	275	50	225	25	50	25	50	150	400	-	-	-	-	-	-
Mankato West HS/Poplar St & Riverfront Dr	AM		-	175	250	125	200	100	225	75	175	75	175	-	-	-		250	525	-	-	-	-	75	175
Signalized Intersection	PM	-	-	175	275	75	225	100	225	250	550	250	550	-	-	1	ı	225	500	-	-	-	-	175	300
Riverfront Dr & Stoltzman Rd	AM	150	250	125	500	125	225	75	150	125	250	25	100	175	275	50	425	75	175	25	75	25	75	25	75
Signalized Intersection	PM	225	250	275	825	125	225	175	250	225	325	175	200	225	275	225	700	75	225	100	125	150	350	100	125
Riverfront Dr & Marshall St	AM	-	-	50	100	25	75	25	75	25	125	-	-	75	150	-	-	-	-	-	-	-	-	-	-
Signalized Intersection	PM	-	-	125	200	75	200	75	200	275	700	-	-	175	375	-	-	0	25	-	-	-	-	-	-

Table C5: 2041 Segment 1 - Option 2A Traffic Operations Analysis

														Move	ment D	elay (s	ec/veh)										
Intersection	Peak Hour		ection * - LOS	E	BL	E	вт	EE	BR	w	BL.	w	ВТ	w	/BR	N	BL	N	вт	N	BR	SI	3L	SI	зт	SE	IR.
Riverfront Dr & Woodland Ave	AM	2	A		-	1	A	0	A	2	A	1	A	1	A	6	A	8	A	2	Α	5	A	7	A		
Stop Controlled	PM	2	A	3	A	0	A	0	A	3	A	1	A	0	A	6	A	8	A	2	Α	6	A	8	A	3	Α
Riverfront Dr & Sibley St	AM	7	A		-	4	A	4	A	8	A	1	A	1	A	39	Е	47	Е	6	Α	20	С	14	В	4	Α
Stop Controlled	PM	6	A	5	A	4	A	4	A	6	A	1	A	1	A	32	D	26	D	2	Α	30	D	33	D	12	В
Mankato West HS/Poplar St & Riverfront Dr	AM	34	D		-	3	A	2	A		-	5	A	2	A		-		-	124	F		-			78	F
Signalized Intersection	PM	37	E		-	4	A	3	A		-	13	В	9	A		-		-	115	F		-		-	321	F
Riverfront Dr & Marshall St	AM	5	A		-	4	A	2	A	13	В	4	A		-	13	В		-	1	A		-		-		-
Signalized Intersection	PM	57	E		-	8	A	4	A	22	C	9	Α		-	51	D		-	1	A		-		-		
				EB	L/T	EB	T/R			W	'BL	WB	T/R									SI	3L	SBL	/T/R		
Riverfront Dr & SB TH 169 Ramps/Owatonna St	AM	14	В	23	C	22	C		-	4	A	4	A		-		-		-		-	13	В	10	В		-
Signalized Intersection	PM	20	С	18	C	17	C		-	5	A	5	A		-		-		-		-	40	Е	22	С		
				EB	L/T	E	ВТ			WBU	J/L/T	W	BR			NBL	/T/R	N	BR								
Riverfront Dr & NB TH 169 Ramps	AM	12	В	9	A	10	В		-	8	Α	16	С		-	20	C	18	C		-		-				-
Signalized Intersection	PM	12	В	7	A	8	A		-	15	C	14	В		-	16	C	15	C		-		-				-
				EBU	/L/T	EB	T/R			WE	BL/T	WB	T/R			N	BL	NBL	/T/R			SB	L/T	SB	Γ/R		
Riverfront Dr & Stoltzman Rd	AM	15	C	13	В	15	С		-	11	В	11	В		-	25	D	17	C		-	9	Α	8	A		-
Signalized Intersection	PM	50	E	43	E	55	F		-	60	F	64	F		-	56	F	29	D		-	36	E	31	D		-

<sup>\*</sup>Delay in seconds per vehicle

Table C6: 2041 Segment 1 - Option 2A Peak Hour Queues By Movement

												Qu	eue Lei	ngths (1	ft)										
Intersection	Peak	Е	BL	E	ВТ	Е	BR	W	BL	W	/BT	W	BR	N	BL	N	ВТ	N	BR	S	BL	S	ВТ	SI	BR
	Hour	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max
Riverfront Dr & Woodland Ave	AM	25	25	25	25	25	25	25	50	25	50	25	50	25	50	25	50	25	50	25	50	25	50	25	50
Stop Controlled	PM	25	25	25	25	25	25	25	50	25	50	25	50	25	50	25	50	25	50	50	75	50	75	50	75
Riverfront Dr & Sibley St	AM	25	50	25	50	25	50	75	175	75	175	25	50	50	350	50	350	25	150	25	50	25	50	25	50
Stop Controlled	PM	25	50	25	50	25	50	50	150	50	150	0	25	25	50	25	50	25	50	25	100	25	100	25	100
Mankato West HS/Poplar St & Riverfront Dr	AM		-	25	175	25	175		-	25	175	25	125		-		-	350	525		-		-	125	300
Signalized Intersection	PM		-	50	150	50	200		-	75	350	100	400		-		-	200	500		-		-	550	650
Riverfront Dr & Marshall St	AM		-	75	150	50 25 7.		25	75	25	125		-	50	100		-	0	0		-		-		-
Signalized Intersection	PM		-	75	150	50	75	100	200	350	775		-	125	250		-	0	0		-		-		-
-		EB	L/T	EB	T/R			W	'BL	WB	BT/R									SI	BL	SBL	/T/R		
Riverfront Dr & SB TH 169 Ramps/Owatonna St	AM	-	125	-	125		-	-	0	-	0		-		-		-		-	-	100	-	75		-
Signalized Intersection	PM	-	75	-	75		-	-	25	-	25		-		-		-		-	-	275	-	150		-
-		EB	L/T	Е	ВТ			WBU	J/L/T	W	/BR			NBL,	/T/R	N	BR								
Riverfront Dr & NB TH 169 Ramps	AM	-	100	-	125		-	-	50	-	150		-	-	25	-	25		-		-		-		-
Signalized Intersection	PM	-	50	-	75		-		150	-	200		-	-	50	-	50		-		-		-		-
_		EBU	/L/T	EB	T/R				SL/T	WB	BT/R			N	BL	NBL	/T/R			SB	L/T	SB	T/R		
Riverfront Dr & Stoltzman Rd	AM	-	125	-	175	-		-	50	-	50		-	-	150	-	100		-	-	0	-	0		-
Signalized Intersection	PM	-	350	-	425			-	325	-	375		-	-	250	-	150		-	-	125	-	125		-

1

Table C7: 2041 Segment 1 - Option 2A Traffic Operations Analysis

			_											Mover	ment D	elay (se	c/veh)										
Intersection	Peak Hour		ection * - LOS	EE	BL	E	ВТ	EI	BR	w	/BL	w	ВТ	w	BR	N	BL	N	ВТ	N	BR	SI	BL	SI	зт	SB	R
Riverfront Dr & Woodland Ave	AM	2	A		-	1	Α	0	A	2	Α	1	A	1	A	6	A	8	Α	2	A	5	Α	7	A	-	
Stop Controlled	PM	2	A	3	Α	0	A	0	A	3	Α	1	A	0	A	6	Α	8	Α	2	A	6	Α	8	A	3	Α
Riverfront Dr & Sibley St	AM	7	A		-	4	A	4	A	8	Α	1	A	1	Α	39	Е	47	Е	6	A	20	C	14	В	4	A
Stop Controlled	PM	6	A	5	A	4	A	4	A	6	A	1	A	1	A	32	D	26	D	2	A	30	D	33	D	12	В
Riverfront Dr & Marshall St	AM	5	A	-		4	Α	2	A	13	В	4	A		-	13	В		-	1	A		-			-	
Signalized Intersection	PM	57	Е			8	A	4	A	22	C	9	A		-	51	D		-	1	A		-		-	-	
				EBI	L/T	EB	T/R			W	BL	WB	T/R									SI	BL	SBL	/T/R		
Riverfront Dr & SB TH 169 Ramps/Owatonna St	AM	14	В	23	C	22	C		-	4	Α	4	A		-		-		-		-	13	В	10	В	-	
Roundabout	PM	20	C	18	C	17	C		-	5	A	5	A		-		-		-		-	40	Е	22	C	-	
				EBI	L/T	EB	T/R			WE	BL/T			W	BR	NB	L/T			NI	BR						
Riverfront Dr & NB TH 169 Ramps	AM	13	В	12	В	15	C		-	6	A		-	15	C	26	D		-	22	C		-			1	
Roundabout	PM	12	В	8	Α	9	A		-	12	В		-	13	В	20	C		-	18	C		-			1	
				EBI	L/T	EB	T/R			WE	BL/T	WE	T/R			N	BL	NBL,	/T/R							SB	R
Mankato West HS/Poplar St & Riverfront Dr	AM	16	C	11	В	14	В		-	16	С	19	C		-	31	D	20	C		-		-			16	C
Roundabout	PM	18	C	9	A	11	В		-	17	С	23	C		-	23	C	13	В		-		-			55	F
				EBI	L/T	EB	T/R			WE	BL/T	WE	T/R			N	BL	NBL,	/T/R			SBI	L/T	SB.	Γ/R		
Riverfront Dr & Stoltzman Rd	AM	11	В	9	Α	9	Α		-	9	Α	9	A		-	16	С	12	В		-	8	A	7	A	-	
Roundabout	PM	27	D	19	C	21	C		-	34	D	37	Е		-	29	D	19	C		-	30	D	26	D	-	

<sup>\*</sup>Delay in seconds per vehicle

Table C8: 2041 Segment 1 - Option 2A Peak Hour Queues By Movement

	Peak											Qu	ieue Lei	ngths (1	ft)										
Intersection	Hour	E	BL	E	ВТ	El	BR	W	/BL	W	/BT	W	'BR	N	BL	N	ВТ	N	BR	SI	BL	SI	3T	SI	BR
	Hour	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max
Riverfront Dr & Woodland Ave	AM	25	25	25	25	25	25	25	50	25	50	25	50	25	50	25	50	25	50	25	50	25	50	25	50
Stop Controlled	PM	25	25	25	25	25	25	25	50	25	50	25	50	25	50	25	50	25	50	50	75	50	75	50	75
Riverfront Dr & Sibley St	AM	25	50	25	50	25	50	75	175	75	175	25	50	50	350	50	350	25	150	25	50	25	50	25	50
Stop Controlled	PM	25	50	25	50	25	50	50	150	50	150	0	25	25	50	25	50	25	50	25	100	25	100	25	100
Riverfront Dr & Marshall St	AM		-	75	150	25	75	25	75	25	125		-	50	100		-	0	0		-		-		-
Signalized Intersection	PM		-	75	150	50	75	100	200	350	775		-	125	250		-	0	0		-				-
		EBI	L/T	EB	T/R			W	BL.	WE	BT/R									SI	BL	SBL	/T/R		
Riverfront Dr & SB TH 169 Ramps/Owatonna St	AM	-	125	-	125		-	-	0	-	0		-		-		-		-	-	100	-	75		-
Signalized Intersection	PM	-	75	-	75		-	-	25	-	25		-		-		-		-	-	275	-	150		-
		EBI	L/T	El	ВТ			WBU	J/L/T	W	/BR			NBL,	/T/R	N	BR								
Riverfront Dr & NB TH 169 Ramps	AM	-	125	-	175		-	-	50	-	225		-	-	50	-	50		-		-		-		-
Signalized Intersection	PM	-	75	-	75		-	-	150	-	175		-	-	50	-	50		-		-				-
		EBI	L/T	EB	T/R			WE	BL/T	WE	BT/R			N	BL	NBL	/T/R							SF	BR
Mankato West HS/Poplar St & Riverfront Dr	AM	-	150	-	200		-	-	100	-	150		-	-	100	-	75		-		-		-	-	50
Roundabout	PM	-	100	-	125		-	-	150	-	225		-	-	50	-	25		-		-			-	175
		EBU	/L/T	EB	T/R			WE	SL/T	WE	BT/R			N	BL	NBL,	/T/R			SB	L/T	SB.	Γ/R		
Riverfront Dr & Stoltzman Rd	AM	-	75	-	75		-	-	25	-	50		-	-	100	-	75		-	-	0	-	0		-
Signalized Intersection	PM	-	150	-	200		-	-	225	-	275		-	-	175	-	100		-	-	100	-	100		-

<sup>\*\*</sup>Maximum delay and LOS on any approach and/or movement

<sup>\*\*\*</sup>Limiting Movement is the highest delay movement.

Table C9: 2041 Segment 1 - Option 3A Traffic Operations Analysis

													Moven	nent Delay	(sec/ve	h)									
Intersection	Peak Hour	Inters Delay		EBU	EBL		EBT		EBR	v	VBL	W	/вт	WBR	ı	NBL	NBT		NBR		SBL		SBT		SBR
SB TH 169 Intersection (Crossover)	AM	15	В	-	-	1	16 I	3	2 A		-	10	В	-		-	-		i		-		-		-
Signalized Intersection	PM	13	В	-	-	1	17 I	3	3 A		-	8	A	-		-	-		-		-		-		-
SB TH 169 Intersection SBL off Ramp	AM	14	В	-	-		3	A	-		-		-	-		-	-		-	-	22	C	-		-
Signalized Intersection	PM	13	В	-	-		3	1	-		-		-	-		-	-		-		19	В	-		-
NB TH 169 Intersection (Crossover)	AM	7	Α	-	-		7	1	-		-	10	В	-		-	-		-	-	-		-		-
Signalized Intersection	PM	9	A	-	-	9	9 1	4	-		-	9	A	-		-	-		-	-	-		-		-
SB TH 169 Intersection WBL onto Ramp	AM	1	Α	-	-		-		-	2	A	1	Α	-		-	-		-	-	-		-		-
	PM	2	A	-	-		-		-	2	A	1	A	-		-	-		-		-		-		-
SB TH 169 SBR off Ramp	AM	2	Α	-	-		-		-		-	2	Α	-		-	-		-	-	-		-		2 A
SB 111 107 SBIC OII Rump	PM	2	A	-	-		-		-		-	2	A	-		-	-		-		-		-		2 A
SB TH 169 Intersection (Off Ramp)	AM	9	A	-	-		-		-		-		-	-		-	-		-	-	9	A	-		6 A
	PM	3	A	-	-		-		-		-		-	-		-	-		-		4	Α	-		2 A
SB TH 169 Intersection (On Ramp)	AM	0	A	-	-		-		-		-		-	-		-	-		-		-			A	1 A
DD 111 107 Intersection (On Tump)	PM	0	A	-	-		-		-		-		-	-		-	-		-		-		0	A	7 A
NB TH 169 Intersection WBR onto Ramp	AM	1	A	-	-		-		-		-	2	A	1 A		-	-		-	-	-		-		-
THE THE PROPERTY OF THE PROPER	PM	2	A	-	-		-		-		-	4	A	1 A		-	-				-		-		-
NB TH 169 Intersection NBR off Ramp	AM	3	Α	-	-	- 1	2	4	-		-		-	-		-	-		9	A	-		-		-
	PM	4	A	-	-		2	4	-		-		-	-		-	-		11	В	-		-		-
NB TH 169 Intersection (On Ramp)	AM	1	A	-	-		-		-		-		-	2 A		-	1	A	-	-	-		-		-
	PM	1	A	-	-		-		-		-		-	1 A		-	0	A	-		-		-		-
NB TH 169 Intersection EBL onto Ramp	AM	8	Α	-	1 A	. 1	11 I	3	-		-		-	-		-	-		-	-	-		-		-
110 111 100 Intersection EBE onto Kamp	PM	12	В	-	1 A	. 1	14 I	3	-		-		-	-		-	-		-		-		-		-
NB TH 169 Intersection NBL off Ramp	AM	2	A	-	-		-		-		-	2	A	-	32	C	-		-	-	-		-		-
Signalized Intersection	PM	2	A	-	-		-		-		-	2	A	-	19	В	-		-		-		-		-
NB TH 169 NBR off Ramp	AM	1	A	-	-		-		-		-		-	-		-	1	A	1	A	-		-		-
TVB TIT TOS TVBR OIT Rump	PM	1	A	-	-		-		-		-		-	-		-	1	A	1	A	-		-		-
Riverfront Dr & Poplar St	AM	2	A	-	-		1 /	4	-		-	1	A	0 A		-	-		-	-	-		-		15 C
Stop Controlled	PM	14	В	-	-	(	0	4	-		-	2	A	1 A		-	-		-		-		-		185 F
Mankato West HS & Riverfront Dr	AM	9	A	-	-	1	10	4	6 A	16	C	1	A	-		-	-		27	D	-		-		-
Signalized Intersection	PM	9	Α	-	-	,	7	Α .	3 A	18	C	2	Α	-		-	-		53	F	-		-		-
Riverfront Dr & Stoltzman Rd	AM	19	В	22 C	22 C	1	15 l	3	8 A	19	В	20	C	5 A	37	D	28	C	11	В	49	D	48	D	15 B
Signalized Intersection	PM	38	D	45 D	53 D	3	31 (	2	11 B	47	D	54	D	18 B	51	D	30	C	10	В	55	Е	50	D	25 C
Riverfront Dr & Marshall St	AM	7	A	-	-		5	1	3 A	15	В	3	A	-	47	D	-		1	A	-		-		-
Signalized Intersection	PM	26	С	-	-		8	A	4 A	26	С	18	В	-	174	F	-		3	A	-		-		-
Riverfront Dr & Sibley St	AM	29	D	-	0 A		2	1	1 A	7	A	0	Α	0 A	77	F	79	F	64	F	17	C	10	A	5 A
Stop Controlled	PM	9	A	-	3 A		1 /	4	1 A	6	A	1	A	0 A	35	Е	48	Е	21	С	28	D	20	С	9 A

<sup>\*</sup>Delay in seconds per vehicle

Table C10: 2041 Segment 1 - Option 3A Peak Hour Queues By Movement

Intersection  SB TH 169 Intersection (Crossover)		Ì											Qu	eue Le	ngths (	ft)											
Intersection	Peak	EB	U	EBL		EBT		EI	BR	W	BL.	W	/BT	W	/BR	N	BL	N	ВТ	N	BR	S	BL	SI	ЗТ	SI	BR
	Hour	Avg	Max	Avg M	ax Av	g M	lax	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max
SB TH 169 Intersection (Crossover)	AM	-		-	125	5 2:	50	0	25		-	25	75		-		-		-		-		-		-		-
Signalized Intersection	PM	-		-	100	) 20	00	0	25		-	50	100		-		-		-		-		-				-
SB TH 169 Intersection SBL off Ramp	AM	-		-	25	7	75		-		-		-		-		-		-		-	200	250		-		-
Signalized Intersection	PM	-		-	25	7	75		-		-		-		-		-		-		-	175	250		-		-
NB TH 169 Intersection (Crossover)	AM	-		-	75	10	00		-		-	75	100		-		-		-		-		-		-		-
Signalized Intersection	PM	-		-	75	10	00		-		-	100	125		-		-		-		-		-		-		-
SB TH 169 Intersection WBL onto Ramp	AM	-		-		-			-	0	0	0	25		-		-		-		-		-		-		-
3B 111 109 Intersection wBL onto Kamp	PM	-		-		-			-	0	50	0	50		-		-		-		-		-		-		-
SB TH 169 SBR off Ramp	AM	-		-		-			-		-	0	0		-		-		-		-		-			25	75
3B III 109 3BR 011 Ramp	PM	-		-		-			-		-	0	25		-		-		-		-		-		-	50	100
SB TH 169 Intersection (Off Ramp)	AM	-		-		-		-			-		-		-	-		-			-	50	500			25	200
SB 111 107 Intersection (Off Ramp)	PM	-		-		-			-		-		-		-		-		-		-	25	150		-	0	50
SB TH 169 Intersection (On Ramp)	AM	-		-		-			-		-		-		-		-		-		-		-	0	0	0	25
SB 111 107 Intersection (On Rump)	PM	-		-		-			-		-		-		-		-		-		-		-	0	0	0	25
NB TH 169 Intersection WBR onto Ramp	AM	-		-		-			-		-	25	125	0	0		-		-		-		-		-		-
TVB 111 105 Intersection WBR onto Ramp	PM	-		-		-			-		-	75	150	0	0		-		-		-	-		-			-
NB TH 169 Intersection NBR off Ramp	AM	-		-	25		75		-		-		-		-		-		-	75	150		-		-		-
TVB TIT TO S Intersection TVBR off Ramp	PM	-		-	25	5	0		-		-		-		-		-		-	125	250		-		-		-
NB TH 169 Intersection (On Ramp)	AM	-		-		-			-		-		-	0	25		-	25	50		-		-		-		-
TVD TIT TO MINORSCOTTON (OIL TURNIP)	PM	-		-		-			-		-		-	0	0		-	0	0		-		-		-		-
NB TH 169 Intersection EBL onto Ramp	AM	-		0 (					-		-		-		-		-		-		-		-		-		-
THE THE TOP INCOME AND THE TOP	PM	-		0 (	200	2:	50		-		-		-		-		-		-		-		-		-		-
NB TH 169 Intersection NBL off Ramp	AM	-		-		-			-		-	0	0		-	0	25		-		-		-		-		-
Signalized Intersection	PM	-		-		-			-		-	0	25		-	25	50		-		-		-		-		-
NB TH 169 NBR off Ramp	AM	-		-		-			-		-		-		-		-	0	0	0	0		-				-
•	PM	-		-		-			-		-		-		-		-	0	0	25	75		-		-		-
Riverfront Dr & Poplar St	AM	-		-	25	_	25		-		-	0	25	0	25		-		-		-		-		-	50	125
Stop Controlled	PM	-		-	25	_	00		-		-	0	50	0	50		-		-		-		-		-	325	550
Mankato West HS & Riverfront Dr	AM	-		-	150	_	75	100	250	75	225	25	100		-		-		-	175	500		-		-		-
Signalized Intersection	PM	-		-	125		25	50	250	75	150	0	50		-		-		-	175	625		-		-		-
Riverfront Dr & Stoltzman Rd	AM	125	250	125 25			75	100	225	50	100	100	225	25	125	200	275	75	450	100	225	25	75	25	100	25	75
Signalized Intersection	PM	200	250	200 25	_	_	25	125	225	175	250	200	300	150	200	200	275	175	550	75	250	100	175	150	325	102	175
Riverfront Dr & Marshall St	AM	-		-	75	_	00	25	125	25	75	50	125		-	75	175		-	0	25		-		-		-
Signalized Intersection	PM	-		-	100	_	_	50	200	50	175	175	350		-	250	550		-	25	225		-		-		-
Riverfront Dr & Sibley St	AM	-		25 5		_	50	25	50	50	175	0	0	0	0	300	525	300	525	300	525	25	50	25	50	25	50
Stop Controlled	PM	-		0 5	0	5	50	0	50	75	200	0	0	0	0	100	325	100	325	100	325	25	75	25	75	25	75

Table C11: 2041 Segment 1 - Option 3B Traffic Operations Analysis

														Moven	nent D	elay (se	c/veh)										
Intersection	Peak Hour		ection * - LOS	Е	BL	E	ЕВТ		EBR		BL.	w	ВТ	w	WBR		BL	NBT		NBR		SBL		SBT		SBR	
Riverfront Dr & Woodland Ave	AM	2	A		-	1	Α	0	A	3	A	1	Α	1	A	6	A	8	Α	2	A	6	A	7	Α		-
Stop Controlled	PM	3	Α	2	Α	0	Α	0	Α	3	Α	1	Α	1	Α	6	A	7	Α	2	Α	6	Α	7	Α	2	Α
Riverfront Dr & Sibley St	AM	8	A		-	6	A	5	Α	11	В	1	A	0	A	35	Е	43	Е	7	A	21	C	16	С	10	В
Stop Controlled	PM	7	A	5	Α	4	Α	4	Α	9	A	2	Α	1	A	26	D	24	C	2	Α	23	C	27	D	9	A
Riverfront Dr & TH 169 SB Ramp	AM	19	В		-	20	C	8	Α	25	C	3	A		-	50	D		-	13	В		-		-		-
Signalized Intersection	PM	32	C		-	49	D	8	A	57	Е	24	C		-	36	D		-	7	A		-		-		-
Riverfront Dr & Owatonna St	AM	4	A	6	A	5	A		-		-	1	A	3	A		-		-		-	18	C		-	4	A
Stop Controlled	PM	10	В	31	D	7	A		-		-	7	A	3	A		-		-		-	159	F		-	10	В
Riverfront Dr & NB TH 169 Ramps	AM	3	A	9	Α	0	A		-		-	4	A	4	A	33	C	18	В	5	A		-		-		-
Signalized Intersection	PM	19	В	22	C	16	В		-		-	12	В	4	A	66	Е	21	C	72	Е		-		-		-
Riverfront Dr & Mankato West HS/Poplar St	AM	21	С	17	В	19	В	16	В	61	Е	14	В	13	В	35	D	34	С	16	В	35	D	33	С	19	В
Signalized Intersection	PM	31	C	36	D	17	В	17	В	59	Е	29	C	31	C	92	F	91	F	42	D	59	Е	61	E	48	D
Riverfront Dr & Stoltzman Rd	AM	18	В	17	В	17	В	10	В	16	В	12	В	3	A	33	C	29	C	10	В	31	C	42	D	9	A
Signalized Intersection	PM	29	С	41	D	32	С	15	В	35	D	31	С	10	В	37	D	36	D	11	В	31	С	50	D	23	C
Riverfront Dr & Marshall St	AM	6	Α		-	4	Α	2	Α	9	Α	3	Α		-	35	D		-	1	Α		-		-		-
Signalized Intersection	PM	12	В		-	8	A	4	A	18	В	10	В		-	45	D		-	1	A		-		-		-

<sup>\*</sup>Delay in seconds per vehicle

Table C12: 2041 Segment 1 - Option 3B Peak Hour Queues By Movement

												Qu	ieue Le	ngths (1	ft)										
Intersection	Peak Hour	EBL		Е	ВТ	E	BR	W	/BL	W	ВТ	W	/BR	N	BL	N	ВТ	NBR		SBL		SBT		SBR	
		Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max										
Riverfront Dr & Woodland Ave	AM	0	25	0	25	0	25	-	-	-	-	-	-	25	75	25	75	25	75	25	50	25	50	25	50
Stop Controlled	PM	0	25	0	25	0	25	-	-	-	-	-	-	25	50	25	50	25	50	50	50	50	50	50	50
Riverfront Dr & Sibley St	AM	25	125	25	125	25	125	75	275	25	25	25	25	75	325	75	325	25	125	25	75	25	75	25	75
Stop Controlled	PM	25	50	25	50	25	50	100	300	25	25	25	25	25	75	25	75	-	-	25	75	25	75	25	75
Riverfront Dr & TH 169 SB Ramp	AM	-	-	225	475	100	550	100	175	-	-	-	-	-	-	-	-	75	550	-	-	-	-	-	-
Signalized Intersection	PM	-	-	300	425	75	375	325	500	-	-	-	-	-	-	-	-	50	300	-	-	-	-	-	-
Riverfront Dr & Owatonna St	AM	0	25	25	250	-	-	-	-	25	75	25	75	-	-	-	-	-	-	50	100	-	-	25	50
Stop Controlled	PM	25	25	25	375	-	-	-	-	25	75	25	75	-	-	-	-	-	-	50	175	-	-	25	100
Riverfront Dr & NB TH 169 Ramps	AM	100	225	-	-	-	-	-	-	50	225	25	200	25	25	25	25	50	75	-	-	-	-	-	-
Signalized Intersection	PM	100	225	175	375	-	-	-	-	125	250	50	200	25	100	25	100	250	900	-	-		-	-	-
Riverfront Dr & Mankato West HS/Poplar St	AM	50	125	175	225	175	225	125	250	175	450	175	450	150	400	150	400	100	150	75	200	75	200	75	200
Signalized Intersection	PM	75	200	225	250	225	250	125	250	350	600	350	600	175	500	175	500	100	150	175	375	175	375	175	375
Riverfront Dr & Stoltzman Rd	AM	25	75	100	200	100	200	50	125	75	175	25	50	175	275	75	425	75	200	25	75	25	125	25	75
Signalized Intersection	PM	125	250	125	325	125	225	150	250	200	275	125	200	200	275	100	350	75	175	75	125	150	325	100	125
Riverfront Dr & Marshall St	AM	-	-	75	200	25	75	25	75	25	125	-	-	75	150	-	-	0	25	-	-	-	-	-	-
Signalized Intersection	PM	-	-	100	250	50	175	50	150	125	300	-	-	125	250	-	-	0	25	-	-	-	-	-	-

Table C13: 2041 Segment 2 - Option 2 Traffic Operations Analysis

														Mover	nent D	elay (s	ec/veh	1)									
Intersection	Peak Hour		ection * - LOS	E	BL	Е	ВТ	E	BR	w	BL	w	/ВТ	w	BR	NI	BL	N	ВТ	N	BR	s	BL	S	вт	SI	BR
Riverfront Dr & Sibley Pkwy	AM	2	A	24	С	1	Α		-		-	2	Α	3	A		-		-		-	25	C		-	5	Α
Signalized Intersection	PM	5	A	39	D	3	A		-		-	5	A	5	A		-		-		-	24	C		-	7	Α
Riverfront Dr & Poplar St/Warren St	AM	26	C	41	D	44	D	18	В	33	C	41	D	14	В	34	C	37	D	19	В	26	C	8	Α	7	Α
Signalized Intersection	PM	30	C	54	D	46	D	29	C	34	C	44	D	22	C	46	D	49	D	19	В	31	C	17	В	15	В
Riverfront Dr & Minnesota St/Cherry St	AM	15	В	41	D	36	D	14	В	40	D	45	D	2	A	15	В	14	В	3	A	46	D	5	A	2	Α
Signalized Intersection	PM	21	C	31	C	22	С	15	В	37	D	35	D	2	Α	33	C	28	C	4	Α	38	D	15	В	7	Α
Riverfront Dr & Main St	AM	14	В	36	D		-		-	29	C		-	16	В	11	В	16	В	8	A	28	C	6	A	2	A
Signalized Intersection	PM	23	C	61	E		-	9	Α	46	D	57	E	34	C	23	C	30	C	15	В	36	D	9	A		-

<sup>\*</sup>Delay in seconds per vehicle

Table C14: 2041 Segment 2 - Option 2 Peak Hour Queues By Movement

												Que	ue Ler	ngths (1	ft)										
Intersection	Peak Hour	Е	BL	EI	вт	E	BR	W	/BL	W	вт	W	BR	N	BL	N	ВТ	N	BR	SI	BL	SI	ВТ	SI	BR
		Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max										
Riverfront Dr & Sibley Pkwy	AM	25	50	25	125	-	-	-	-	25	100	25	50	-	-	-	-	-	-	25	75	-	-	25	50
Signalized Intersection	PM	25	75	50	125	•	-	-	-	50	150	25	50	-	-	-	-	-	-	50	100	1	-	25	50
Riverfront Dr & Poplar St/Warren St	AM	50	125	50	125	50	125	50	125	50	200	50	200	50	225	275	650	225	625	50	125	75	200	75	200
Signalized Intersection	PM	75	200	100	200	100	200	100	175	100	225	100	225	75	250	300	625	250	600	100	225	200	350	200	350
Riverfront Dr & Minnesota St/Cherry St	AM	50	125	50	100	50	100	125	275	125	275	25	25	25	150	225	375	50	300	50	100	75	175	25	75
Signalized Intersection	PM	75	150	50	125	50	125	200	350	200	350	25	50	75	200	300	375	75	325	75	100	175	200	25	100
Riverfront Dr & Main St	AM	25	25	25	25	25	25	75	175	-	-	75	150	25	50	200	475	50	350	50	125	50	100	50	100
Signalized Intersection	PM	25	75	25	75	25	75	150	225	-	-	75	175	25	225	375	775	125	400	50	150	75	225	75	225

Table C15: 2041 Segment 3 - Options 2A, 3A and 4A Traffic Operations Analysis

														Mover	nent D	elay (se	ec/veh)										
Intersection	Peak Hour	Inters Delay	ection * - LOS	EE	BL	EI	ВТ	E	BR	w	BL	w	ВТ	w	BR	N	BL	N	ВТ	N	BR	s	BL	s	ВТ	SI	BR
Riverfront Dr & Plum St	AM	10	В	-			-		-	30	С			17	В	11	В	12	В	6	A	17	В	5	A	3	A
Signalized Intersection	PM	13	В	41	D		-		-	38	D		-	23	C		-	16	В	10	В	27	C	8	A		
Riverfront Dr & Elm St	AM	6	A	24	С	34	C	9	A	25	C		-	11	В	14	В	7	Α	7	A	22	C	3	A	3	Α
Signalized Intersection	PM	10	В	47	D	53	D	27	C	49	D	39	D	26	C	38	D	10	В	9	A	29	C	7	Α	6	Α
Riverfront Dr & Madison Ave	AM	14	В	39	D	47	D	6	A	40	D	37	D	9	A	15	В	11	В	5	A	13	В	12	В	4	A
Signalized Intersection	PM	24	С	60	E	57	E	8	A	44	D	38	D	10	В	22	C	24	С	12	В	23	C	24	C	5	Α

<sup>\*</sup>Delay in seconds per vehicle

Table C16: 2041 Segment 3 - Options 2A, 3A and 4A Peak Hour Queues By Movement

												Qu	eue Ler	ngths (1	ft)										
Intersection	Peak Hour	EI	BL	EE	3T	EI	BR	W	BL	W	ВТ	W	BR	N	BL	N	ВТ	N	BR	S	BL	S	ВТ	SI	BR
		Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max										
Riverfront Dr & Plum St	AM	-	-	-	-	-	-	25	50	25	50	50	125	25	25	150	500	25	250	50	100	50	175	50	175
Signalized Intersection	PM	25	50	25	50	25	50	25	75	25	75	50	100	-	-	200	575	50	300	75	150	75	200	75	200
Riverfront Dr & Elm St	AM	25	75	25	75	25	75	25	50	25	50	25	50	25	50	75	225	75	225	25	50	50	175	50	175
Signalized Intersection	PM	50	125	50	125	50	125	50	75	50	75	50	75	25	50	100	325	100	325	25	75	125	325	125	325
Riverfront Dr & Madison Ave	AM	50	125	50	125	50	125	100	175	-	-	75	175	75	175	100	275	100	275	50	175	50	225	50	225
Signalized Intersection	PM	75	175	75	175	75	125	200	325	-	-	100	175	75	275	200	375	200	375	75	150	125	300	125	300

Table C17: 2041 Segment 3 - Options 2B, 3B and 4B Traffic Operations Analysis

														Mover	nent D	elay (se	ec/veh)										
Intersection	Peak Hour	Delay	ection * - LOS	El	BL	EI	ВТ	E	BR	w	BL	w	ВТ	w	BR	N	BL	N	ВТ	N	BR	s	BL	s	ВТ	SI	BR
Riverfront Dr & Plum St	AM	9	A				-		-	35	D		-	17	В	6	A	11	В	6	A	18	В	5	A	4	A
Signalized Intersection	PM	14	В	38	D		-		-	41	D		-	22	C		-	16	В	10	В	28	С	8	A		-
Riverfront Dr & Rock St	AM	6	A	20	С	20	C	11	В	27	C		-	13	В	16	В	8	A	9	Α	20	С	2	Α	2	Α
Signalized Intersection	PM	9	A	51	D	45	D	33	C	53	D	41	D	22	C	37	D	12	В	10	В	31	C	4	Α	2	Α
Riverfront Dr & Madison Ave	AM	14	В	44	D	48	D	6	A	41	D	36	D	8	A	14	В	10	В	5	Α	14	В	12	В	4	A
Signalized Intersection	PM	24	C	73	E	59	E	8	A	43	D	37	D	11	В	24	C	24	C	14	В	25	C	24	C	9	Α

<sup>\*</sup>Delay in seconds per vehicle

Table C18: 2041 Segment 3 - Options 2B, 3B and 4B Peak Hour Queues By Movement

												Qu	eue Lei	ngths (1	ft)										
Intersection	Peak Hour	EI	BL	E	ВТ	EI	BR	W	'BL	W	ВТ	W	BR	N	BL	NI	ВТ	N	BR	S	BL	S	ВТ	SI	BR
		Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max										
Riverfront Dr & Plum St	AM	-	-	-	-	-	-	25	50	25	50	50	125	0	25	125	425	25	125	50	100	50	150	50	150
Signalized Intersection	PM	25	50	25	50	25	50	25	75	25	75	50	100	-	-	225	600	25	300	75	175	75	225	75	225
Riverfront Dr & Rock St	AM	25	75	25	75	25	75	25	75	25	75	25	75	25	50	75	200	75	200	25	50	75	200	75	200
Signalized Intersection	PM	50	125	50	125	50	125	50	100	50	100	50	100	25	50	100	300	100	300	25	100	125	250	125	250
Riverfront Dr & Madison Ave	AM	50	150	50	150	50	125	100	200	75	175	75	175	75	175	100	225	100	225	50	125	100	225	50	200
Signalized Intersection	PM	75	150	75	150	75	150	175	350	150	325	75	175	100	300	225	425	225	425	75	150	175	325	125	275

Table C19: 2041 Segment 5 - Option 1 Traffic Operations Analysis

Intersection		Peak Hour		ection * - LOS					Moven	nent D	elay (se	c/veh)				
					EBL/	T/R			NI	ВТ	NI	3R	SBI	L/T		
Riverfront Dr & TH 14 EB Ran	np	AM	14	В	28	D		-	9	A	7	A	11	В		-
Roundabout		PM	14	В	17	C		-	20	С	9	A	8	A		-
							WBL	/T/R	NB	L/T			SE	BT.	SE	3R
Riverfront Dr & TH 14 WB Ran	np	AM	13	В	-	-	13	В	9	A			20	С	9	A
Roundabout		PM	17	C	-	-	14	В	21	C			10	В	7	Α

<sup>\*</sup>Delay in seconds per vehicle

Table C20: 2041 Segment 5 - Option 1 Peak Hour Queues By Movement

Intersection	Peak Hour		N	laximum Que	ue Lengths (ft	:)	
		EBL/T/R		NBT	NBR	SBL/T	
Riverfront Dr & TH 14 EB Ramp	AM	200	-	75	25	100	-
Roundabout	PM	125	-	200	50	50	-
			WBL/T/R	NBL/T		SBT	SBR
Riverfront Dr & TH 14 WB Ramp	AM	-	50	75	-	150	25
Roundabout	PM	-	50	250	-	50	25

Table C21: 2041 Segment 5 - Option 2 Traffic Operations Analysis

	Peak	Inters	ection											Move	ment D	elay (s	ec/vel	1)								
Intersection	Hour		* - LOS	Е	BL	Е	ВТ	E	BR	W	/BL	W	/ВТ	W	/BR	N	BL	N	ВТ	N	BR	9	BL	s	ВТ	SBR
EB TH 14 Intersection (Crossover)	AM	13	В	-		-		-		-		-		-		-		15	В	6	A	-		16	В	-
Signalized Intersection	PM	17	В	-		-		-		-		-		-		-		16	В	7	Α	-		28	C	-
EB TH 14 Intersection SBL off Ramp	AM	6	A	19	В	-		-		-		-		-		-		2	A	-		-		-		-
Signalized Intersection	PM	6	A	18	В	-		-		-		-		-		-		2	A	-		-		-		-
WB TH 14 Intersection (Crossover)	AM	11	В	-		-		-		-		-		-		-		10	A	-		-		12	В	-
Signalized Intersection	PM	11	В	-		-		-		-		-		-		-		8	A	-		-		17	В	-
EB TH 14 Intersection WBL onto Ramp	AM	3	Α	-		-		-		-		-		-		-		-		1		3	Α	3	A	-
EB 1H 14 linersection wBL onto Ramp	PM	7	Α	-		-		-		-		-		-		-		-		-		4	Α	7	A	-
EB TH 14 SBR off Ramp	AM	4	A	-		-		7	A	-		-		-		-		-		-		-		3	A	-
EB 1H 14 SBK on Kamp	PM	3	A	-		-		6	A	-		-		-		-		-		-		-		1	A	-
EB TH 14 Intersection (Off Ramp)	AM	0	A	0	A	-		0	Α	-		-		-		-		-		-		-		-		-
EB 1H 14 littersection (On Kamp)	PM	1	Α	0	Α	-		1	Α	-		-		-		-		-		-		-		-		-
EB TH 14 Intersection (On Ramp)	AM	3	A	-		0	A	4	A	-		-		-		-		-		-		-		-		-
EB 1H 14 littersection (On Kamp)	PM	4	A	-		0	A	4	A	-		-		-		-		-		-		-		-		-
WB TH 14 Intersection WBR onto Ramp	AM	2	A	-		-		-		-		-		-		-		-		-		-		2	A	1 A
WB 1H 14 Intersection WBR onto Ramp	PM	1	Α	-		-		-		-		-		-		-		-		-		-		1	A	0 A
WB TH 14 Intersection NBR off Ramp	AM	3	A	-		-		-		-		-		1	A	-		10	В	-		-		-		-
WB 1H 14 linersection NBR off Ramp	PM	2	A	-		-		-		-		-		1	A	-		14	В	-		-		-		-
WB TH 14 Intersection (On Ramp)	AM	2	A	-		-		-		-		1	A	3	A	-		-		-		-		-		-
WB 1H 14 linersection (Oli Kamp)	PM	2	Α	-		-		-		-		0	A	5	A	-		-		-		-		-		-
WB TH 14 Intersection EBL onto Ramp	AM	2	A	-		-		-		-				-		1	A	2	A	-		-		-		-
wb 1ft 14 intersection EBL onto Ramp	PM	3	Α	-		-		-		-				-		1	A	5	A	-		-		-		-
WB TH 14 Intersection NBL off Ramp	AM	8	A	-		-		-		23	С	-		-		-		-		-		-		2	A	-
Signalized Intersection	PM	11	В	-		-		-		19	В	-		-		-		-		-		-		4	A	-
WD TH 14 NDD off Down	AM	1	A	-		-		-		0	A	-		0	A	-		-		-		-		-		-
WB TH 14 NBR off Ramp	PM	1	Α	-		-		-		1	A	-		0	A	-		-		-		-		-		-

<sup>\*</sup>Delay in seconds per vehicle

Table C22: 2041 Segment 5 - Option 2 Peak Hour Queues By Movement

_	Deal											Qu	eue Le	ngths (	ft)										
Intersection	Peak	Е	BL	Е	ВТ	Е	BR	W	/BL	W	/BT	W	BR	N	BL	N	ВТ	N	BR	SI	BL	S	ВТ	SE	BR
	Hour	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max	Avg	Max										
EB TH 14 Intersection (Crossover)	AM		-		-		-		-		-				-	150	275	100	175		-	200	250		-
Signalized Intersection	PM		-						-		-		-		-	225	300	150	175		-	175	250		-
EB TH 14 Intersection SBL off Ramp	AM	75	150		-		-		-		-		-		-	0	0		-		-		-		-
Signalized Intersection	PM	100	175						-		-		-		-	25	75		-		-		-		-
WB TH 14 Intersection (Crossover)	AM		-		-		-		-		-		-		-	100	150		-		-	150	200		-
Signalized Intersection	PM		-		-		-		-		-		-		-	100	150		-		-	125	175		-
ED TH 14 Internation WDL anta Dance	AM		-		-		-		-		-		-		-		-		-	25	175	50	200		-
EB TH 14 Intersection WBL onto Ramp	PM		-		-		-		-		-		-		-		-		-	25	125	50	150		-
ED TH 14 CDD off Dome	AM		-		-	100	200		-		-		-		-		-		-		-	25	100		-
EB TH 14 SBR off Ramp	PM		-		-	75	175		-		-		-		-		-		-		-	25	75		-
EB TH 14 Intersection (Off Ramp)	AM	0	0		-	0	0		-		-		-		-		-		-		-		-		-
EB 1H 14 Intersection (Off Ramp)	PM	0	0			0	0		-		-		-		-		-		-		-		-		-
EB TH 14 Intersection (On Ramp)	AM		-	0	0	25	75		-		-		-		-		-		-		-		-		-
EB 1H 14 littersection (On Kamp)	PM		-	0	0	25	75				-		-		-		-		-		-		-		-
WB TH 14 Intersection WBR onto Ramp	AM		-				-				-		-		-		-		-		-	25	150	0	0
WB 1H 14 linersection WBR onto Ramp	PM		-		-		-		-		-		-		-		-		-		-	25	75	0	0
WB TH 14 Intersection NBR off Ramp	AM		-		-		-		-		-	0	0		-	50	125		-		-		-		-
WB 111 14 Intersection (VBK off Kamp	PM		-		-		-		-		-	0	0		-	25	75		-		-		-		-
WB TH 14 Intersection (On Ramp)	AM		-		-		-		-	0	0	50	100		-		-		-		-		-		-
WB 111 14 intersection (on Ramp)	PM		-		-		-		-	0	0	50	75		-		-		-		-		-		-
WB TH 14 Intersection EBL onto Ramp	AM		-		-		-		-				-	0	0	25	100		-		-		-		-
WB 111 14 Intersection EBE onto Ramp	PM		-		-		-		-				-	0	0	75	200		-		-		-		-
WB TH 14 Intersection NBL off Ramp	AM		-		-		-	100	175		-		-		-		-		-		-	25	50		-
Signalized Intersection	PM		-		-		-	100	175		-				-		-		-		-	25	25		-
WB TH 14 NBR off Ramp	AM		-		-		-	0	0		-	0	0		-		-		-		-		-		-
WB III 14 NBK OII Rainp	PM		-		-		-	0	0		-	0	0		-		-		-		-		-		-

Appendix H: Evaluation Matrices

#### Evaluation Matrix Riverfront Drive Corridor Study January 2017



											***
Segment 1 - V	Voodland Aven	ue to Sibley Parkway			Traditional Sig	US 10 Inalized Corridor	69 Interchange to	Stoltzman Road C ut Corridor	Options Interchange N	Modifications	
Goal	Objectives	Performance Measure		No Build	Option 1A	Option 1B	Option 2A Three	Option 2B Four	Option 3A  Diverging	Option 3B Loop	Notes
			AM	44 D	Triple Lefts  11 B	Double Lefts  13 B	Roundabouts 9 A	Roundabout 12 B	Diamond 6 A	Ramp 10 A	1.51.63
		Vehicle Delay on Riverfront	PM	31 C	16 B	23 C	18 B	14 B	9 A	19 B	
		Intersection Delay by Type:  Private Drive (YMCA/School Driveway, Cub)	AM	194 F	49 D	38 D	185 F	23 C	28 C	26 C	
	Address peak hour back ups at Poplar Street and		PM AM	47 D 107 F	40 D 30 C	50 D 53 D	66 F 13 B	26 C 23 C	45 D 10 A	47 D 18 B	
	between Owatonna St and Marshall St.	Interchange Ramp	PM AM	48 D 48 D	41 D 12 B	37 D 13 B	30 C 10 A	29 C 14 B	6 A 9 A	30 C 11 B	
		Public Street (Woodland, Sibley, Poplar, Stotlzman, Marshall)	PM	39 D	17 B	25 C	25 C	14 B	13 B	20 B	
		Segment Delay (veh-hr)	AM PM	230 177	57 90	71 125	64 124	51 79	60 96	51 108	
		Score		1	7	5	3	9	9	9	
		Travel Time (Eastbound)	AM PM	253 194	153 185	163 209	147 173	149 152	168 181	165 215	
	Ensure mobility and reliability on Riverfront	Travel Time (Westbound)	AM PM	779 266	135 168	138 220	134 320	141 176	161 216	142 194	
Goal A:	Drive and to Stoltzman	Average Travel Speeds (Eastbound)	AM	18 16	21 17	20 15	19 16	18 18	17 15	21 15	
Provide efficient ehicle and freight		Average Travel Speeds (Westbound)	PM AM	6	25	24	21	19	17	23	
obility and access.		Score	PM	14 1	20 9	15 6	9 5	16 8	13 6	17 7	
		Vehicle Delay on Side Streets	AM PM	102 F 66 E	24 C	33 C	37 D	14 B 27 C	21 C 29 C	19 B 33 C	
		Number of Access Points on Riverfront by Type:	PM	66 E	34 C	41 D	56 E	27 C	29 C	33 C	
		Full Access 3/4 Access		12 0	12 0	11	11 0	12 0	12 0	12 0	
	Provide reasonable and	Right-In/Right-Outs		0	0	0	1	0	0	0	
	responsible access.	Roundabouts		0	0	0	3	4	0	0 Poor - Requires new	
		Provides reasonable access to existing businesses		Good	Good	Fair - 3/4 Poplar (U-turns at Signal)	Fair - RABs facilitate U-turns	Fair - RABs facilitate U-turns	Poor - Requires school site reconfiguration	Hubbell and	
		Score		2	9	8	7	8	5	Owatonna 5	
	Ensure freight mobility and access.	Accommodates Truck Staging Needs		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Not a differentiator for this segment.
		Score Coal A Score (100 Points Total)		- 12	- 02						
		Goal A Score (100 Points Total)  Crash Rate Reduction (Cumulative)		- 13	48%	63 52%	50 60%	58%	67	70 56%	
	Study opportunities to address crash issues.	Vehicle to vehicle conflict points		113	122	88	49	51	62	93	
Goal B:		Vehicle to pedestrian conflict points  Score		72 1	75 3	55 7	38 10	39 10	43 9	72 6	
fely accommodate all users.	Study opportunities to address vehicles	Dandung daring you idea to office as less to a fife at		Poor	Poor	Fair	Good	Good	Good	Poor	
	traveling above posted speed limits.	Roadway design provides traffic calming effect									
		Score Goal B Score (100 Points Total)		1 10	1 20	5 60	10 100	10 100	10 95	1 35	
		Number of lanes to cross in single stage:		5	5	5	2	2	2	5	A single stage of crossing is the distance
		US 169 SB Ramp/Owatonna Street Poplar Street		5	6	6	2	2	2	5	pedestrians need to cross to access the other of the roadway or pedestrian refuge such as
		Stoltzman Road  Mankato West to Cub Foods		5 N/A	6 2	5 2	2	2	5 3	6 2	central median. All lanes were included if intersection is signal controlled.
		Distance to cross Riverfront Drive:  US 169 SB Ramp/Owatonna Street		98'	98'	98'	28' & 24'	28' & 24'	24' & 32'	27' & 39'	
		US 169 NB Ramp		-	-	-	27' & 28'	-	24' & 24'	-	
		Poplar Street Stoltzman Road		70' 90'	79' 90'	36' & 25' 90'	- 27' & 28'	27' & 28' 27' & 28'	97'	70' 90'	
		Mankato West to Cub Foods Sidewalk width under 169/60 Bridge		N/A 6'/6'	24' & 23' 8'/6'	24' & 23' 8'/10'	24' & 24' 6'/6'	24' & 24' 6'/6'	24' & 36' 6'/6'	24' & 23' 8'/10'	Not a differentiator
		Boulevard under 169/60 Bridge		Yes - 7'	Yes - 7'/5'	Yes - 7' both	Yes - 10' both	Yes - 10' both	Yes - 5' one side	Yes - 7' both	
Goal C: rovide an inviting	Provide safe pedestrian crossings near Sibley St,	Mid-Block Median Width (School to Cub)  Ped. LOS-Mid-Block Crossing: School to Cub		N/A	14'	14	12	12'	23	14'	
nd safe pedestrian Invironment both	Poplar St, and Stoltzman Road.	Crosswalk Markings and Signage Only:									
along and across Riverfront Drive.		Average Pedestrian Delay (sec/ped)		213	202.1	290.1	275.4	213	118.8*	212	LOS F -Delay exceeds tolerance level, high cl of risk-taking.
Kiverironi Drive.											Evaluation assumes the application of cross markings and signage only.
		•Level of Service (A-F)		F	F	F	F	F	F*	F	*Only north side of Riverfront Drive Evalua South side is signal controlled intersection
		Rectangular Rapid Flashing Beacon:									
		Average Pedestrian Delay (sec/ped)		9.3	9.2	8.3	8.6	9.3	4.9*	10.2	LOS B -Occasionally some delay due to conflict traffic.
											LOS C - Delay noticeable to pedestrians, but inconvienencing.
		•Level of Service (A-F)		В	В	В	В	В	A*	С	Evaluation assumes the application of Rectangular Rapid Flashing Beacon.
				_							*Only north side of Riverfront Drive Evalua South side is signal controlled intersection
		Score Goal C Score (100 Points Total)		1 10	3 30	5 50	9 90	9 90	8 80	5 50	
Goal D:		Accommodates bicycles at primary crossings:									
Support bicycle	Accommodate connections to										
onnections across iverfront Drive to	Minneopa Trail/West Mankato Trail crossing.	Woodland Avenue		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Not a differentiator for this segment. All op could be work with a grade separation of the
esignated parallel bike routes and	Walkato Hall Crossing.	Poplar Street		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Mankato Trail.
regional trails.		Score Goal D Score (100 Points Total)									
		Court of Storie (100 ) of the stories									
Goal E:		Provides a future street connection through the Cub Foods retail									The future road connection through Cub is
pports future land	Supports future land use	complex to support future redevelopment opportunities between Sibley Pkwy and Linder Ave		No	No	Yes	Yes	Yes	Yes	No	required with Options 1A and 3B since Poplaremains full access with these options
use and redevelopment											
plans.		Impacts to Developable Sites on Cub Retail Complex property  Score		None 3	Low 4	Low 8	High 5	High 5	Low 8	Low 4	
		Goal E Score (100 Points Total)		30	40	80	50	50	80	40	
		Number of Acquisitions (full-partial)		0	0/6	0/4	4/10	5/10	2/5	0/11	
		Risk of Impacts to Historic/Cultural Resources		Low	Medium	Low	High	High	Medium	Medium	
	and impacts to	or imposts to instancy cultural resources		_UW	cuidiii	LOW	riigii	rugli	MEGIGIII		
Goal F:	historic/cultural and natural resources.	Risk of Impacts to Natural Resources		N/A	Low	Low	Low	Low	Low	Low	Not a differentiator.
Provide infrastructure											
improvements mpatible with the		Impacts to City street network west of Hwy 169 (Hubbell/Owatonna St)  Score		No 10	No 8	No 9	No 3	No 1	Yes - Owatonna 4	Yes - Hubbell 6	
storic and natural	Minimize Cost.	Estimated Cost Score		10	\$ \$700,000 9	\$1,000,000 8	\$5,000,000 5	\$6,400,000 4	4 \$4,500,000 6	\$4,300,000 6	
environment.		Provide additional space for streetscape elements.		N/A	No	Yes - median	Yes - median and roundabouts	Yes - median and roundabouts	Yes - median	No	
	Level of Community Familiarity/Acceptan	Level of community familiarity/acceptance		N/A	Medium	High	High	High	Low	High	
	Miles Inc. ! !					1			1		
	Minimize Long Term Maintenance Costs.	Level of long-term maintenance costs.  Score		N/A 1	Medium - 5 signals	Medium - 6 signals	Low - 2 signals	Low - 2 signals	High - 8 signals	Medium - 6 signals	

# Evaluation Matrix Riverfront Drive Corridor Study February 2017



segment 2 - S	ibiey Parkway	to Veteran's Memorial Bridge				4-Lane wi		afety and F cements	edestrian	3-Lane w Safety Pedes	y and	
Goal	Objectives	Performance Measure		No	Build	Option 17			on 1B xpansion	Option 2	2A Lane	Notes
		Vehicle Delay on Riverfront	AM	14	В	14	В	14	В	14	В	
		Maximum Queue	PM	13	В	13	В	13	В	24	С	
	Address peak hour back		AM	NBT	275	NBT	275	NBT	275	NBT	800	
	ups at Warren St,	Poplar St/Warren St	PM	NBT	275	NBT	275	NBT	275	NBT	950	
	Cherry St, & Main St.	Minnesota St/Cherry St	AM PM	NBT NBT	225 300	NBT NBT	225 300	NBT NBT	225 300	NBT NBT	375 375	
		A	AM	NBT	150	NBT	150	NBT	150	NBT	500	
		Main St	PM	NBT	250	NBT	250	NBT	250	NBT	800	
Goal A:		Score	AM	14	9	14	В	14	В В	28	C	
Provide efficient		Vehicle Delay on Side Streets	PM	18	В	18	В	18	В	34	С	
vehicle and freight nobility and access.		Number of Access Points on Riverfront by Type:				1		T		,		
ioomit, and accessi	Provide reasonable and	Full Access 3/4 Access			0	(	)		5 1	5		
	responsible access.	Right-In/Right-Outs			0	(	)		)	C	)	
		Roundabouts			0	(	)	1	0	С	)	
		Provides reasonable access to existing businesses		G	ood	Go	od	Fa	air	Fa	ir	
		Score			8	8	3	!	9	7	,	
	Ensure freight mobility and access.	Accommodates Truck Staging Needs		١	/es	Ye	es	Ye	es	Ye	es	Not a Determining Factor
		Score										
		Goal A Score (100 Points Total)			85	8	5	9	0	7(	0	
<b>Goal B:</b> Safely	Study opportunities to address travel speeds above the posted speed	Roadway design provides traffic calming effect		N	I/A	N	0	N	lo	Ye	s*	*Reduction to one northbound lane ma encourage slower speeds.
Safely accommodate all	limits.											encourage slower speeds.
users.		Score  Goal B Score (100 Points Total)			1 10	1			.0	7		
		Provides Pedestrian Refuge at Primary Crossings					•					
		Provides Pedestrian Refuge at Primary Crossings     Warren Street		Ye	s - 7'	Yes	- 7'	Yes	- 7'	Yes	- 7'	
		•Cherry Street			s - 7'	N		Yes		Yes		
		Orivic Center Drive		Yes	i - 15'	Yes		No	- 4'	No		*Need to relocate refuge crossing to the no
	Provide safe pedestrian crossings near Warren,	Mid-Block to Civic Center Front Door		`	/es	N	0	N	lo	Ye	s*	Would not line up with Civic Center front d
	Cherry, Hy-Vee and the Civic Center.	Main Street		Ye	s - 7'	N	0	No	- 4'	No	- 4'	
	one center	Distance to Cross at Riverfront Drive (ft)  •Warren Street			71	6	1	6	1	6	1	
		•Cherry Street			83	80			15	81		*Single stage crossing (no refuge)
		•Civic Center			65	6		6		57		*Single stage crossing (no refuge)
		Main Street     Score			80	74	l* I		9* <b>4</b>	71		*Single stage crossing (no refuge)
Goal C:		Provides Pedestrian facilities in missing areas:							•			
upport an inviting nd safe pedestrian		Between Main and Walnut		Yes	s - <5'	Yes - 8' V	V, 6' Blvd	Yes - 8' \	V, 6' Blvd	Yes - 8' W	, 10' Blvd	Not a Differentiator
environment both		Between Civic Center Dr and Cherry St			No	Yes - 8' V	V, 4' Blvd	Yes - 8' \	V, 6' Blvd	Yes - 8' W	/, 7' Blvd	
along and across Riverfront Drive		Number of Lanes to Cross in a Single Stage		Min	Max	Min	Max	Min	Max	Min	Max	
		Warren Street		2	3	2	3	2	3	2	3	
	Improve Walkability	Cherry Street		2	4	6	6	2	4	2	4	
	along and across the corridor.	Civic Center		2	3	5	5	2	3	1	3	
		•Main Street		2	4	6	6	2	4	1	3	
		Pedestrian LOS Score for Segment										
		Cherry to Civic Center Plaza (South Side)		1	N/A		:		С	C	)	Factors effecting results include number
												adjacent thru-lanes, proportion of sidewa adjacent to buildings, sidewalk width, et
		Score			2				В	9		
Goal D:		Goal C Score (100 Points Total)			50	5	U		0	7:	•	
Support bicycle	Accommodate on-street bicycle connections	Accommodates facilities at Cherry Street		١	⁄es	Ye	es	Y	es	Ye	es	No changes affect on-street bicycle facilitie. Cherry Street across Riverfront Drive.
	bicycle connections											Charry Street across rivermont brive.
		C										-
connections across Riverfront Drive to designated parallel		Score  Goal D Score (100 Points Total)										_
Riverfront Drive to		Goal D Score (100 Points Total)										Unknown until we meet with the Ve-
Riverfront Drive to lesignated parallel bike routes and Goal E:	Supports future land											Unknown until we meet with Hy-Vee
diverfront Drive to esignated parallel hike routes and Goal E: upports future land	Supports future land	Goal D Score (100 Points Total)		ı	N/A	N	0	Ye	25*	N	0	*Road ROW enxroaches onto the Hy-Vee pro
iverfront Drive to esignated parallel bike routes and  Goal E: apports future land use and redevelopment	Supports future land	Goal D Score (100 Points Total)  Supports Hy-Vee Redevelopment Plans		N	N/A	N	0	Ye	:5*	N	0	*Road ROW enxroaches onto the Hy-Vee pro
iverfront Drive to esignated parallel bike routes and Goal E: pports future land use and	Supports future land	Goal D Score (100 Points Total)  Supports Hy-Vee Redevelopment Plans  Impacts to Developable Space on Hy-Vee Property  Score			N/A 0		0		n:s*	N.		*Road ROW enxroaches onto the Hy-Vee pro
tiverfront Drive to esignated parallel bike routes and Goal E: upports future land use and redevelopment	Supports future land	Goal D Score (100 Points Total)  Supports Hy-Vee Redevelopment Plans  Impacts to Developable Space on Hy-Vee Property										*Road ROW enxroaches onto the Hy-Vee pro
Riverfront Drive to designated parallel bike routes and Goal E: upports future land use and redevelopment plans.	Supports future land use	Goal D Score (100 Points Total)  Supports Hy-Vee Redevelopment Plans  Impacts to Developable Space on Hy-Vee Property  Score					)	-			)	*Road ROW enxroaches onto the Hy-Vee propushing sidewalks further onto the prope
Goal F: Provide	Supports future land use  Avoid property impacts and impacts to	Supports Hy-Vee Redevelopment Plans Impacts to Developable Space on Hy-Vee Property  Score  Goal E Score (100 Points Total)  Number of Partial Acquisitions			0	(	)		1	0		*Road ROW enxroaches onto the Hy-Vee propushing sidewalks further onto the prope
Goal F: Provide infrastructure	Supports future land use  Avoid property impacts and impacts to	Supports Hy-Vee Redevelopment Plans Impacts to Developable Space on Hy-Vee Property  Score  Goal E Score (100 Points Total)		N	<b>0</b> 0	(	)		0	C		*Road ROW enxroaches onto the Hy-Vee propushing sidewalks further onto the prope  *Option 1B encroaches on the Union Deproperty. The Union Depot is listed on the National Registry of Historic Places.
iverfront Drive to esignated parallel bike routes and  Goal E: pports future land use and redevelopment plans.  Goal F: Provide infrastructure improvements	Avoid property impacts and impacts to historic/cultural and natural resources.	Supports Hy-Vee Redevelopment Plans Impacts to Developable Space on Hy-Vee Property  Score  Goal E Score (100 Points Total)  Number of Partial Acquisitions  Risk of Impacts to Historic/Cultural Resources		7	0 0 1/A	LC LC	o) w w	High	0 1 2h*	C Lo	w w	*Road ROW enxroaches onto the Hy-Vee propushing sidewalks further onto the property.  *Option 1B encroaches on the Union Deproperty. The Union Depot is listed on
Goal F: Provide infrastructure improvements propartible with the istoric and natural	Avoid property impacts and impacts to historic/cultural and natural resources.	Supports Hy-Vee Redevelopment Plans Impacts to Developable Space on Hy-Vee Property  Score  Goal E Score (100 Points Total)  Number of Partial Acquisitions  Risk of Impacts to Historic/Cultural Resources		7	<b>0</b> 0	( Lc	) ) w w	High	1 1 2h*	C Lo	w w	*Road ROW enxroaches onto the Hy-Vee propushing sidewalks further onto the prope  *Option 1B encroaches on the Union Deproperty. The Union Depot is listed on the National Registry of Historic Places.  There is no potential for impacts to natural
Goal F: Provide infrastructure improvements on the test of the province of the test of the	Avoid property impacts and impacts to historic/cultural and natural resources.	Supports Hy-Vee Redevelopment Plans Impacts to Developable Space on Hy-Vee Property  Score  Goal E Score (100 Points Total)  Number of Partial Acquisitions  Risk of Impacts to Historic/Cultural Resources  Risk of Impacts to Natural Resources  Score  Estimated Cost  Score		7	0 0 1/A 1/A	Lc Lc \$1,17'	0 0 0 0 5,000	High Lc \$2,13	0 1 1 gh* bw 4 5,000	C Lo Lo \$450,	w w 0 0 0000	*Road ROW enxroaches onto the Hy-Vee propushing sidewalks further onto the prope  *Option 1B encroaches on the Union Deproperty. The Union Depot is listed on the National Registry of Historic Places.  There is no potential for impacts to natural
Goal F: Provide infrastructure improvements on attraction and redevelopment plans.	Avoid property impacts and impacts to historic/cultural and natural resources.  Minimize Cost.	Supports Hy-Vee Redevelopment Plans Impacts to Developable Space on Hy-Vee Property  Score  Goal E Score (100 Points Total)  Number of Partial Acquisitions  Risk of Impacts to Historic/Cultural Resources  Risk of Impacts to Natural Resources  Score  Estimated Cost		7	0 0 1/A 1/A 10 -	Lc Lc \$1,17:	0 0 0 0 5,000	High Lc \$2,13	0 1 gh* ow 4 5,000	Lo Lo \$450,	w w 0 0 0000	*Road ROW enxroaches onto the Hy-Vee propushing sidewalks further onto the proper pushing sidewalks further onto the proper sidewalks further onto the proper sidewalks further onto the Property. The Union Depot is listed on the National Registry of Historic Places.  There is no potential for impacts to natural
Goal F: Provide infrastructure improvements propartible with the istoric and natural	Avoid property impacts and impacts to historic/cultural and natural resources.  Minimize Cost.  Enhance Community Identity.	Supports Hy-Vee Redevelopment Plans Impacts to Developable Space on Hy-Vee Property  Score  Goal E Score (100 Points Total)  Number of Partial Acquisitions  Risk of Impacts to Historic/Cultural Resources  Risk of Impacts to Natural Resources  Score  Estimated Cost  Score		P P	0 0 1/A 1/A 10 -	Lc Lc \$1,17'	0 w w 0 5,000 5	High Lc \$2,13	0 1 1 gh* bw 4 5,000	C Lo Lo \$450,	w w 0000000000000000000000000000000000	*Road ROW enxroaches onto the Hy-Vee propushing sidewalks further onto the proper pushing sidewalks further onto the proper property. The Union Depot is listed on the National Registry of Historic Places.  There is no potential for impacts to natural
Goal F: Provide infrastructure improvements papatible with the istoric and natural environment.	Avoid property impacts and impacts to historic/cultural and natural resources.  Minimize Cost.	Supports Hy-Vee Redevelopment Plans Impacts to Developable Space on Hy-Vee Property  Score  Goal E Score (100 Points Total)  Number of Partial Acquisitions  Risk of Impacts to Historic/Cultural Resources  Risk of Impacts to Natural Resources  Score  Estimated Cost  Score  Goal F Score (100 Points Total)		h i	0 0 1/A 1/A 10 -	( Lc Lc Lc L 1 1 51,17!	0 0 0 0 5,000 5 5	\$2,13 \$2,13	0 1 gh* bw 4 5,000 3	0 Lo Lo 450, \$450, 8	w w w 0 0 0000 3 0 0 0 0 0 0 0 0 0 0 0 0	*Road ROW enxroaches onto the Hy-Vee propushing sidewalks further onto the proper pushing sidewalks further onto the proper property. The Union Depot is listed on the National Registry of Historic Places.  There is no potential for impacts to natural resources

#### Evaluation Matrix Riverfront Drive Corridor Study February 2017



egment 3 - V	/eteran's Mem	orial Bridge to Madison Avenue				Spot Safety and Enhancements		Spot Safety and Enhancements		dians and Parking Sides	3-Lane Median	is and turn lanes	
	Objectives	Performance Measure		No Build	Option 1A Primary Int. Plum & Elm	Option 1B Primary Int. Plum & Rock	Option 2A Primary Int. Plum & Elm	Option 2B Primary Int. Plum & Rock	Option 3A Primary Int. Plum & Elm	Option 3B Primary Int. Plum & Rock	Option 4A Primary Int. Plum & Elm	Option 4B Primary Int. Plum & Rock	Notes
	Provide adequate	Vehicle Delay on Riverfront	AM	7 A	7 A	7 A	8 A	8 A	8 A	8 A	8 A	8 A	
	Drive.		PM	10 A	10 A	10 A	15 B	16 B	15 B	16 B	15 B	16 B	
		Score	AM	10 14 B	10 14 B	10 14 B	9 19 B	9 19 B	9 19 B	9 19 B	9 19 B	9 19 B	
Goal A:	Provide reasonable and		PM	18 B	18 B	18 B	29 C	30 C	29 C	30 C	29 C	30 C	
vide efficient	responsible access.	Maintains Left Turn Access to Businesses/Parking  Number of Left Turn Access to Businesses/Parking	North	Yes 17	Yes 17	Yes 17	Yes 17	Yes 17	No 16	No 16	No 14	No 14	
icle and freight ility and access.		Score	South	19 <b>10</b>	19 10	19 10	19 9	19 9	16 8	16 8	14 7	14 7	
mey and access.	Ensure freight mobility	Accommodates Truck Staging Needs											
	and access.	Maintains trucks ability to stage on Riverfront Dr.*  Score		Yes 10	Yes 10	Yes 10	No 8	No 8	No 8	No 8	No 8	No 8	*Trucks have been observed staging in lanes on Riverfront Drive. The abil trucks to perform this illegal movement is reduced in some options.
		Goal A Score (100 Points Total)		100	100	100	87	87	83	83	80	80	
		Crash Rate Reduction (Cumulative)		-	0%	0%	47%	47%	47%	47%	47%	47%	
Goal B: accommodate	address crash issues.	winimizes parallel parking maneuvers conflicting with thru traffic		Good	Good	Good	Fair	Fair	Poor	Poor	Fair	Fair	
sers (vehicles,		Score Roadway design provides traffic calming effect		No	3 No	3 No	9 Yes	9 Yes	7 Yes	7 Yes	9 Yes	9 Yes	
eight, transit, pedestrians,	address travel speeds above the posted speed	keduced Number of Lanes		No	No	No	Yes	Yes	Yes	Yes	Yes	Yes	
bicycles)	limits.	Bumpouts into Riverfront/Medians Score		Yes/No 1	Yes/No 1	Yes/No 1	Yes/No 6	Yes/No 6	Yes/Yes 10	Yes/Yes 10	Yes/Yes 8	Yes/Yes 8	
		Goal B Score (100 Points Total)		20	20	20	75	75	85	85	85	85	
		Provides Pedestrian Refuge at Primary Crossings		No	No	No	No	No	Yes - 13'	Yes - 13'	No - 4'	No - 4'	
		Number of Lanes to Cross in a Single Stage (West/East)  •Rock Street		4	4	4	3	3	3	3	3	3	
		•Elm Street •Plum Street		4 5	4 5-6	4 5-6	3 3-4	3 3-4	3 3-4	3 3-4	3 3-4	3 3-4	
		Crossing Distance - Decreased (Y/N) / New Distance (ft)  •Rock Street		N/A 50	No 50	No 50	Yes 38	Yes 38	Yes 38	Yes 38	Yes 46	Yes 46	
		•Elm Street •Plum Street		N/A 50 N/A 55	No 50 No 75	No 50 No 75	Yes 38 Yes 50	Yes 38 Yes 50	Yes 38 Yes 50	Yes 38 Yes 50	Yes 46 Yes 50	Yes 46 Yes 50	
	Accommodates pedestria	Midth of Sidewalks		10'	10'	10'	16'	16'	11-12'	11-12'	12'	12'	
	other alternative for	Pedestrian LOS Score for Segment		South North	South North	South North	South North	South North	South North	South North	South North	South North	
Goal C: port an inviting	n			сс	с с	сс	D D	D D	D C	D C	в с	в с	Delica   D
safe pedestrian ironment both		Unsignalized Crossing LOS: Elm/Rock											Unsignalized Crossing LOS Thresholds:
ng and across		Crosswalk Markings and Signage Only:											A - 0-5s (Delay)_Usually no conflicting Traffic B - 5-10s _Occasionally some delay
Riverfront Drive		Level of Service (A-F)		F	F	F	F	F	E	E	F	F	C - 10-20s_Delay noticeable to peds, but not inconveniencing D - 20-30s_Delay noticeable/irritating, increased change of risk-taking
		Rectangular Rapid Flashing Beacon:											E - 30-45s_Delay approaches tolerance level, risk taking likely F - >45s_Delay exceeds tolerance level, high chance of risk-taking
		Level of Service (A-F)     Score		A 4	A 4	A 4	8 8	8	A 9	A 9	A 7	A 7	
		Provides On-Street Parking:					-				-	-	
	Assess need for additional on-street	•One / Both Sides		One	One	One	One	One	Both	Both	One	One	
	parking and accessibility to available off-street			35	35	35	35	35	66	65	35	35	
	public parking areas.	Maintains full access to public parking lots		Yes	Yes	Yes	Yes	Yes	No	No	No	No	
		Score		8	8	8	8	8	9	9	8	8	
		Goal C Score (100 Points Total)		60	60	60	80	80	90	90	75	75	
Goal D: Support bicycle connections across Riverfront Drive to designated parallel bike routes and regional trails.  Goal E: upports future land use and	Accommodate future or street bicycle facility crossing at Elm Street o other Alternatives.	n Accommodates future on-street bicycle crossing at Elm Street r		Poor	Fair	Poor	Good	Fair	Good	Fair	Good	Fair	Options with a signal at Elm rated higher along with those with fewer traffic to cross.
		Score  Goal D Score (100 Points Total)		3 30	6 60	3 30	9	6 60	9 90	6 60	9 90	6 60	
	Support Old Town redevelopment	Supports Redevelopment Plans		Poor	Fair	Fair	Good	Good	Good	Good	Good	Good	Not a differentiator
	strategies.	Impacts to Developable Space Score		No 1	No 6	No 6	No 9	No 9	No 9	No 9	No 9	No 9	No Impacts to Developable Space.
development		Goal E Score (100 Points Total)		10	60	60	90	90	90	90	90	90	
Goal F: Provide infrastructure improvements ompatible with the	Avoid property impacts and impacts to	Number of Acquisitions (full-partial)		None	None	None	None	None	None	None	None	None	No Impacts to Properties. Proposed Construction is within ROW.
	historic/cultural properties in Old Town and Zonta Gardens.	Risk of Impacts to Historic/Cultural Resources		Low	Low	Low	Low	Low	Low	Low	Low	Low	No Impacts to Historical/Cultural Resources. Proposed Construction is v ROW.
	and zonta gardens.	Risk of Impacts to Natural Resources		Low	Low	Low	Low	Low	Low	Low	Low	Low	No Impacts to Natural Resources. Proposed Construction is within RO
ric and natural nvironment.	Minimize Cost.	Score Estimated Cost			\$325,000	\$575,000	\$550,000	\$800,000	\$550,000	\$800,000	\$525,000	\$775,000	Not a differentiator
		Score Goal F Score (100 Points Total)		10 100	8 80	7	7 70	5 50	7 70	5	7 70	5 50	
		Provides Additional Space for Streetscape Elements.		Poor	Poor	Poor	Good	Good	Good	Good	Fair	Fair	A rating of good was given if there was additional sidewalk space, median bumpouts for streetscape amenities.
Goal G: nce Community	Support Identity of Old Town District	Accommodates Events/Public Gathering Spaces		Poor	Poor	Poor	Good	Good	Fair	Fair	Fair	Fair	
Identity		Level of community familiarity/acceptance		N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	
		Level of long-term maintenance costs.		Low	Low	Low	Low	Low	Medium	Medium	Medium	Medium	
		Score		3	3	3	10	10	8	8	7	7	

# Evaluation Matrix Riverfront Drive Corridor Study February 2017



Segment 4 - N	Madison Avenu	e to Good Council Drive														
Goal	Objectives	Performance Measure	No Build		Opt 3rd Ave Realigne	ion 1 d to Adams Stree	et	18		ion 2 aligned Chestnut	St			tion 3 d Ave Connection		Notes
		Number of Access Points on Riverfront by Type:														
	Provide reasonable and	Full Access	14	9 6				9								
	responsible access.	Right-In/Right-Out	0							6				5		
		Provides reasonable access to existing businesses	Good	Fair -	some full accesses conve		out access	Fair - sor		rted to right-in / right-o	ut access	Fair - so		erted to right-in / right-	out access	
Goal A:		Score	5			8				7				8		
Provide efficient vehicle and freight mobility and access.	Ensure freight mobility and access & consider network connectivity options between Madison/3rd Ave	Accommodates truck movements from 3rd Ave to Riverfront Dr	Good		Fair - relocation to Ad	ams St or Madison Ave	2	Poor - shift to Maxfield or Chestnut for full access					Good - closest to			
		Score	10			8				5				10		
		Goal A Score (100 Points Total)	75			30				60				90		
		Riverfront Crossing at Lafayette St / 3rd Ave	No Build	Option 1 (On Layo	Option A (With Median)	Option B (With Median)	Option C	Option 2 (On Layout)	Option A (With Median)	Option B (With Median)	Option C	Option 3 (On Layout	Option A (With Median)	Option B (With Median)	Option C	
	Provides Pedestrian Crossing Opportunities,	•Improves Pedestrian Crossings	N/A	Yes	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
	Particularly Near	Provides Pedestrian Refuge	No	9' Median	13' Median	13' Median	15' Median	9' Median	13' Median	13' Median	15' Median	9' Median	13' Median	13' Median	15' Median	
<b>Goal B:</b> Safely	Schools.	Number of Lanes to Cross in a Single Stage	Min Max 5 5	Min Max		Min Max 1 1	Min Max 1 1	Min Max 2 2	Min Max 2 2	Min Max 1 1	Min Max 1 1	Min Max 2 2	Min Max 2 2	Min Max 1 1	Min Max 1 1	
accommodate all		Distance to Cross Riverfront (Per Stage) (ft)	61 61	24 24	24 24	15 15	17 17	24 24	24 24	15 15	17 17	24 24	24 24	15 15	17 17	
users (vehicles,		Score	1	7	8	9	9	7	8	9	9	7	8	9	9	
freight, transit, pedestrians, bicycles)	Ensure Compatibility with Transit Development Plan  Accommodates bus transit access on designated routes		Yes	No - Right-in / right-out (medians) access will reroute transit route 3 entering Riverfront from Lime St.			No - Right-in / right-out (medians) access will reroute transit route 3 entering Riverfront from Lime St.				t No - Right-in / right-out (medians) access will reroute transit route 3 entering Riverfront from Lime St.				Bus route 3 traverses from Madison Avenue to Lime	
bicyclesy		Score	10	7	7	7	7	7	7	7	7	7	7	7	7	
	Study opportunities to address vehicles traveling above posted speed limits.	Roadway design provides traffic calming effect	Poor	Poor	Fair	Good	Good	Poor	Fair	Good	Good	Poor	Fair	Good	Good	
		Score	1	2	6	9	10	2	6	9	10	2	6	9	10	
		Goal B Score (100 Points Total)	40	53	70	83	87	53	70	83	87	53	70	83	87	
<b>Goal F:</b> Provide infrastructure improvements	Avoid impacts to property and historic/natural resources.	Number of Acquisitions (full-partial)	0		2 full -	4 partial			1	full			3 full	- 1 partial		
compatible with the historic and natural environment.	Minimize Costs								0,000			\$67				
		Score	10	6	6	6	6	4	4	4	4	3	3	3	3	
		Goal F Score (100 Points Total)	100	60	60	60	60	40	40	40	40	30	30	30	30	
Goal G:		Provide additional space for streetscape elements.	No	9' Median, No Bl	/d 13' Median, No Blvd	13' Median, 9' Blvd	13'Median, 6' Blvd	9' Median, No Blvd	13' Median, No Blvd	13' Median, 9' Blvd	13'Median, 6' Blvd	9' Median, No Blvd	13' Median, No Blvo	d 13' Median, 9' Blvd	13'Median, 6' Blvd	
	identity in ways			1			1						1		1	
<b>Enhance Community</b>		Level of long-term maintenance costs.	Low	Medium	Medium	Medium	Medium	Medium	Medium	Medium	Medium	Medium	Medium	Medium	Medium	
Enhance Community Identity	compatible with	Level of long-term maintenance costs.  Score  Goal F Score (100 Points Total)	2 20	Medium 6 60	Medium 8	10 100	Medium 9	Medium 6	Medium 8 80	10 100	Medium 9 90	Medium 6	Medium 8 80	10 100	Medium 9 90	

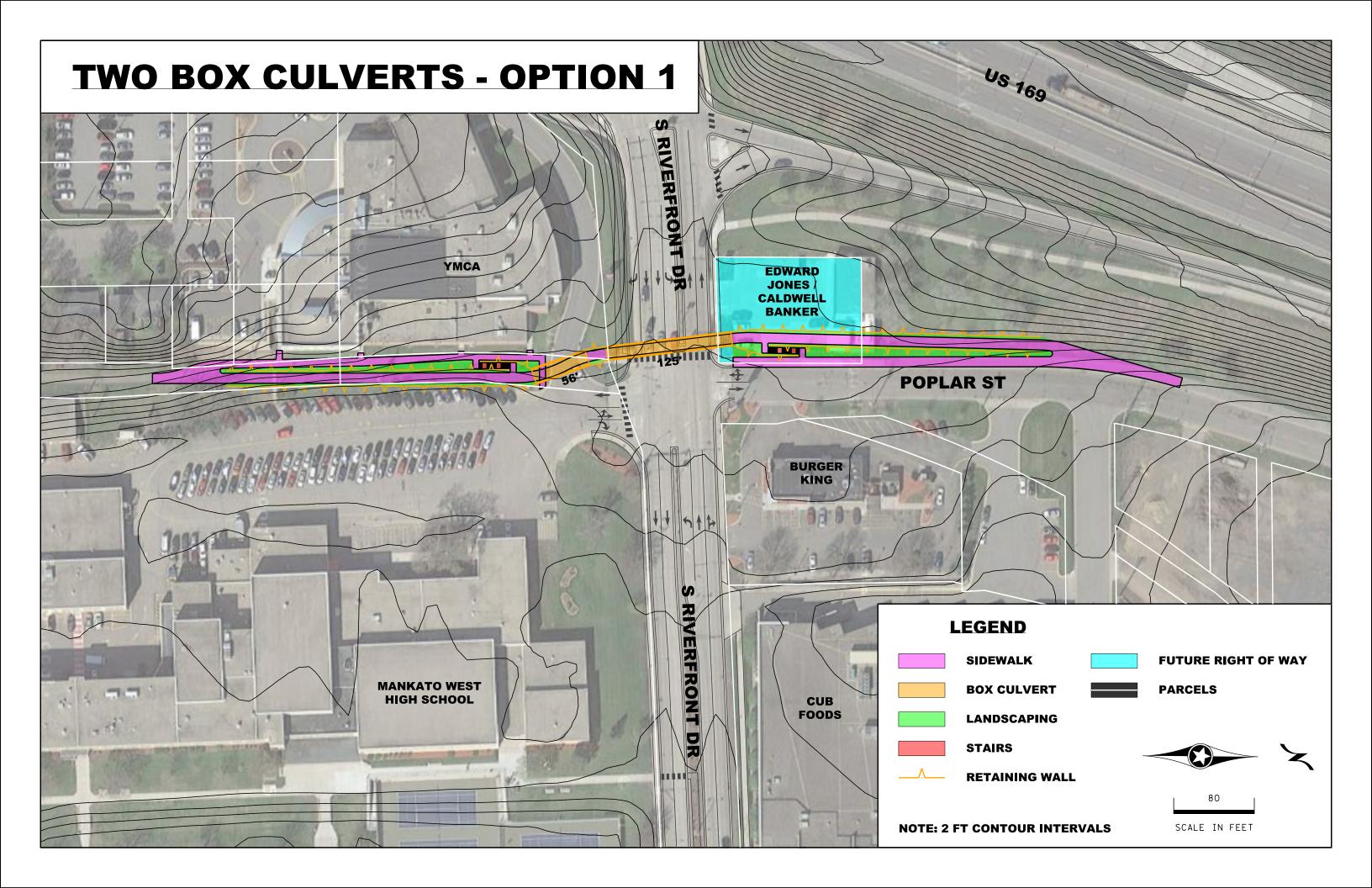
### **Evaluation Matrix** Riverfront Drive Corridor Study January 2016

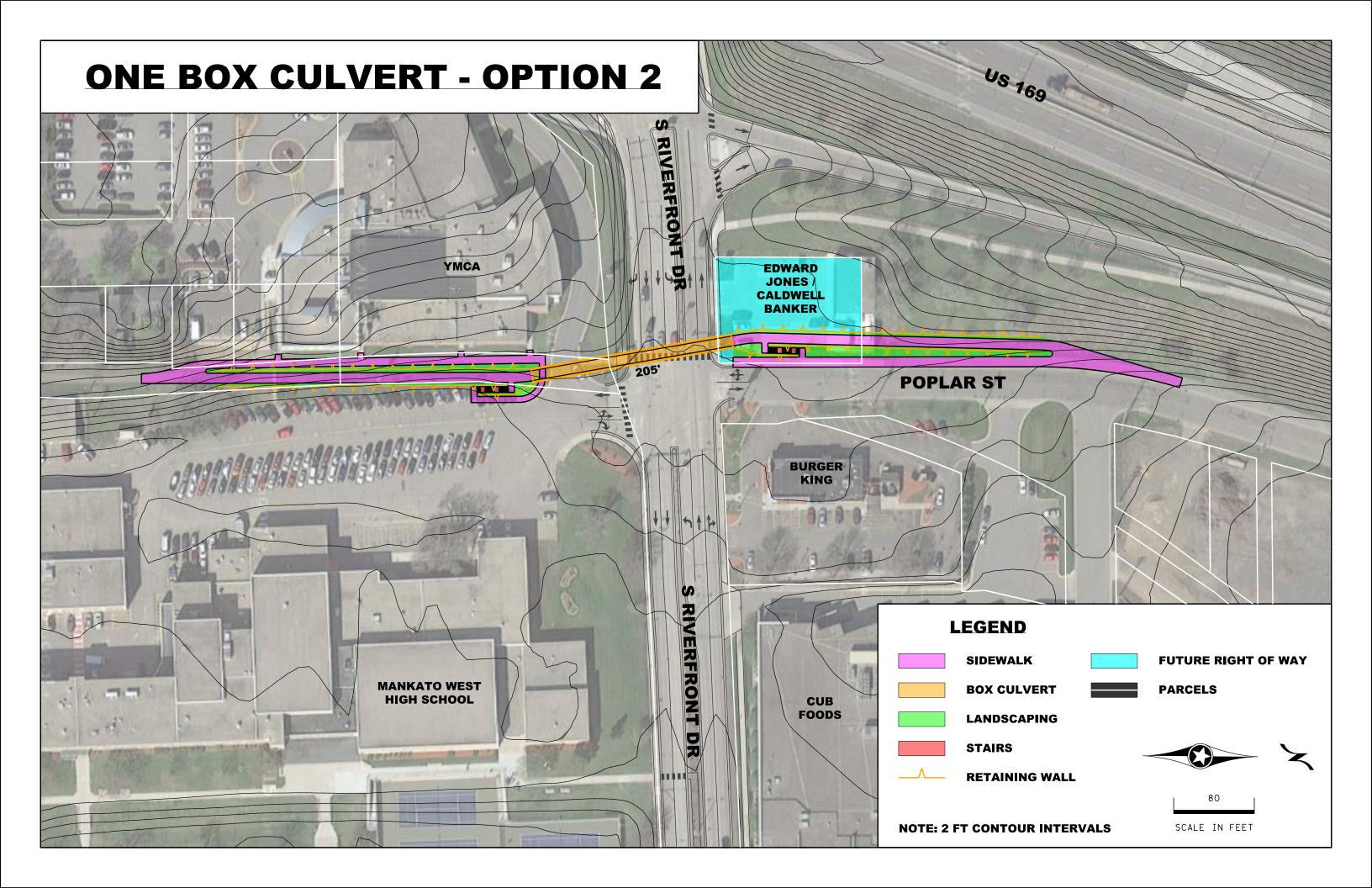


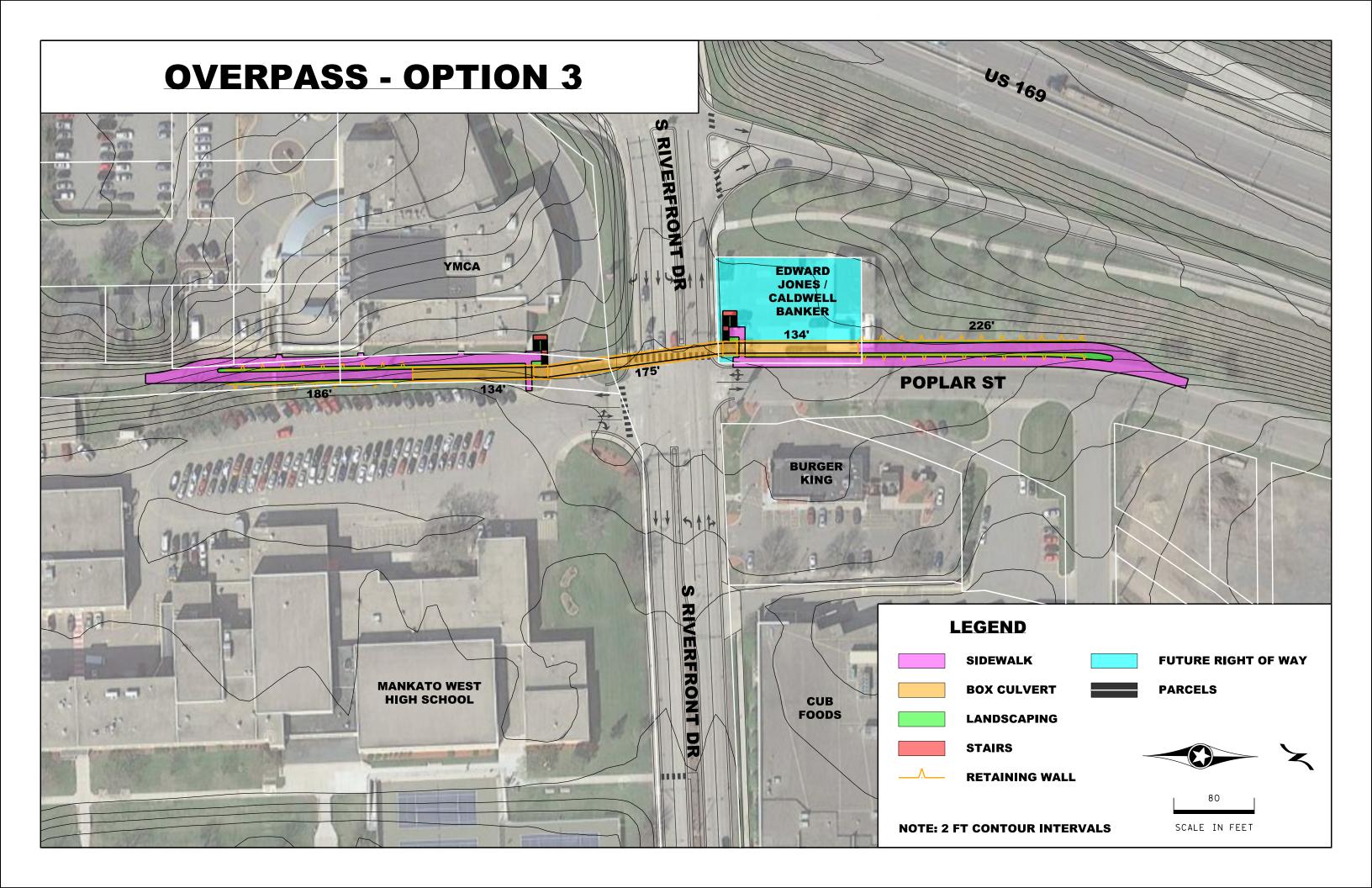
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										<b>V</b>	
Segment 5 - F	lighway 14 Int	erchange				Hwy	/ 14 Inter	section Op			
Goal	Objectives	Performance Measure		Nlo	Ruild	Option 1 -		Option Diverg		Notes	
Guai	Objectives	Performance Measure		INO	No Build		dabouts	Diverg		Notes	
		Vehicle Delay on Riverfront	AM	2	А	23	С	6	Α		
	Address peak hour back ups at Highway 14 ramp		PM	2	А	16	С	9	Α		
	intersections.	Vehicle Delay on Ramps	AM	99	F	20	С	5	Α .		
		Score	PM	193	1		<b>9</b>	10	A		
	Ensure mobility and	Maximum Intersection Capacity Utilization		0.54			1.01	0.73	}		
	reliability on Riverfront  Drive and to Highway	Maximum Volume to Capacity Ratio		3	3.01	(	0.99	0.78	;		
Goal A:		Score			1		6	10			
Provide efficient vehicle and freight		Number of Access Points on Riverfront by Type:			2		2	2		I	
mobility and access.	Provide reasonable and	Full Access 3/4 Access			0		0	3			
	responsible access.	Right-In/Right-Outs			0		0	0			
		Roundabouts			0		0	0			
		Score								Not a differentiator for this segment.	
	Ensure freight mobility and access.	Accommodates Truck Staging Needs		N	N/A		N/A			Not a differentiator for this segment.	
	and access.	Score								Not a differentiator for this segment.	
		Goal A Score (100 Points Total)			10	_	75				
		Crash Rate Reduction (Cumulative)			-		57%	57%			
	Study opportunities to address crash issues.	Vehicle to vehicle conflict points			32		7	12			
	,	Vehicle to pedestrian conflict points			4		6	7			
Goal B:	Study opportunities to	Score			1		9	8			
Safely accommodate all users.	address vehicles traveling above posted speed limits.	Roadway design provides traffic calming effect  Score			No Yes 1 10		Yes	Yes			
	opeca milita.										
		Goal B Score (100 Points Total)			10		95				
		Number of lanes to cross in single stage:								A single stage of crossing is the distance pedestrians need to cross to access the other side of the roadwa	
		Good Counsel Drive			2-3		2-3			or pedestrian refuge such as central median. All lanes were included if intersection is signal controlled.	
		TH 14 EB		2-3		1		1			
Goal C:	Provide safe pedestrian	Distance to cross Riverfront Drive:								controlled.	
Provide an inviting	crossings particularly	Good Counsel Drive			72'		72'	72'			
and safe pedestrian environment both	near schools.	TH14 EB			83'		16', 22', 18'	4 Stage - 16',1			
along and across		Sidewalk width									
Riverfront Drive.		North/South side at Good Counsel Drive			6'		6'	6'			
		North/South side at TH 14 WB  Boulevard Width Under Bridge			8' N/A		18'	8' 5'		Provides buffer from traffic.	
		Score			1		9	8		Trovides butter from trutter	
		Goal C Score (100 Points Total)			10		90	80			
Goal D:	Accommodate future	Accommodates bicycles at primary crossings:									
Support bicycle connections across Riverfront Drive to	trail crossing to Minnesota River Trail.	To Minnesota River Trail		1	No		Yes	Yes			
designated parallel		Score									
bike routes and		Goal D Score (100 Points Total)									
Goal E: Supports future land use and redevelopment	Supports future land use	Consitent with future land use plans		١	Yes		Yes	Yes		Not a differentiator for this segment.	
plans.		Score									
		Goal E Score (100 Points Total)									
		Number of Acquisitions (full-partial)			0		0	0			
	Avoid impacts to historic properties and natural resources.	Risk of Impacts to Historic/Cultural Resources		N	lone	N	lone	None	9		
Goal F: Provide	- 1 2 655d. 663.	Risk of Impacts to Natural Resources		N	lone	N	lone	None	e		
infrastructure improvements		Score					I		Not a differentiator for this segment.		
compatible with	Minimize Cost.	Estimated Cost Score			10	\$1,3	00,000	\$1,600,0	000		
community and the historic and natural	Enhance Community	Provides space for additional streetscape elements.			No		Yes	Yes			
environment.	Identity.  Level of Community  Familiarity/Acceptance	Level of community familiarity/acceptance.		N	N/A	H	High	Low	,	Roundabouts are familiar and accepted in community. Diverging diamond is a new concept.	
	Minimize long term maintenance costs.	Level of long-term maintenance costs.		N	N/A	ı	Low	High	1	2 signal systems with diverging diamond option.	
		Score			1		10	7			
		Goal F Score (100 Points Total)			55		90	70		1	

Appendix I: Grade Separated Pedestrian Crossings at the West Mankato Trail







Appendix J: Implementation Table with Public Prioritization Results

## Implementation Priorities – Input from April 20, 2017 Open House

	Project #	Project Description	Estimated Cost**	Comments	Public Input on Timeframe Priorities		
	1A OR	Triple Lefts at TH 169 South Ramp, Add Right Turn Lane on Riverfront to YMCA/School, Two-Stage Pedestrian Crossing (School to Cub Foods), Add Right Turn Lane on Stoltzman	\$700,000 - \$1.0M	Could choose Either 1A OR 1B. Need to further investigate the feasibility of adding a 3rd lane under the TH 169 bridge with Option 1A. Requires coordination with MnDOT and Blue Earth County.	00000000		
		Double Lefts at TH 169 South Ramp, 3/4 at Poplar Street with Median, Add Right Turn Lane on Stoltzman					
	Cub Foods New Public Street	Establish a public street connection from Riverfront Drive to Linder Ave through the Cub Foods parking lot drive lane	\$175,000	Requires close coordination with property owner.			
	2	Lane alignment modifications to the Riverfront Drive intersections with Warren Street and Cherry Street	\$40,000	Opportunity to study needs at the Warren/Poplar St intersection with Riverfront Drive further during the 2017-2018 Warren Street studies.			
	2	Installation of protected lefts on Warren Street/Popular Street and Southbound Riverfront Drive at Cherry Street	\$35,500	Opportunity to study needs at the Warren/Poplar St intersection with Riverfront Drive further during the 2017-2018 Warren Street studies.			
	2, 3	Test a 3-Lane on Riverfront Drive from Cherry Street to Vine Street Test 2nd Street enhancements (bump-outs, marked crosswalks, etc.) at same time as Riverfront Drive	\$55,000 - \$65,000	Estimated cost is to test a 3-lane. The lower cost represents using paint and the upper range for using removable tape striping. Testing 2nd St enhancements would be an add \$900 per bump-out and \$900 per crosswalk.			
	3	Add wayfinding for public parking locations and public spaces.	\$6,000	\$500 per sign	0000000000		
	5	Construct roundabout at TH 14 North Ramp	\$750,000	Requires coordination with MnDOT and Blue Earth Count			
		Subtotal	\$2.0 - \$2.5M				
	2-1A	4-Lane narrow median to add sidewalk on east side of Riverfront Drive					
	OR 2-2	3-lanes to add sidewalk on east side of Riverfront Drive.	\$1.2M OR \$450,000	3-lane in Segment 2 would need to be paired with 3-lane in Segment 3. Both options maintain full access at Civic Center Plaza/back parking lot to Hy-Vee.	000000		
ı	All Segment 3 Options	Remove free-right at Plum Street	\$60,000				
Air	4-Lane with Pedestrian Flasher (RRFB) at Rock St or Elm St (wherever traffic signal is not present)						
۱	OR 3-2, 3-2B, 3-3, 3-4	-2B, One of 2-1 and Ontions (if comfortable after 2-1 and test period)		These costs include removing the free right at Plum. Consider an overhead RRFB system if a 4- lane is maintained on Riverfront Drive. A ground mounted RRFB system could be considered with a 3-lane.			
		Subtotal					
	All Segment 1 Options	Extend a public street the proposed Cub Food public street extension at Linder Ave to Sibley Parkway	\$300,000	Development driven with former City Public Works site			
	All Segment 1 Options	Grade Separated Trail Crossing near Poplar Street	\$1- \$1.5M	Reevaluate need after short-term improvements to Segment 1 of Riverfront Drive are implemented.			
ı	1-2A	Roundabouts at TH 169 Ramp intersections with Riverfront Drive and Stoltzman Road.	\$5M	Will require right-of-way acquisition and coordination with adjacent property owners			
ı	1-28	Roundabout at TH 169 South Ramp and Stoltzman Road. Six-legged roundabout with 169 North Ramp and Poplar Street.	\$6.5M	Not supported by YMCA or School	5050		
н	1-3A	Diverging Diamond at TH 169; right-in/right-out at Poplar St/YMCA and School access	\$4.5M	Not supported by YMCA or School			
	1-3B	Add a loop ramp to the TH 169 interchange; raise Riverfront Drive to accommodate.	\$4.5M	Will require additional city street network enhancements.			
	2-18	4-Lane Shift West	\$2.2M	Feasible only if Hy-Vee decides to expand on a new building footprint. Allows wide center median to remain.			
	4	Median on Riverfront Drive at Adams Street with trail extension to 3rd Avenue	TBD	Trail extension addresses need identified in Safe Routes to School Plan.			
	4	3rd Avenue/Madison - 3rd Avenue Realignment to 4th Leg of Madison Ave/Riverfront Dr intersection	TBD	Development driven in conjunction with Coughlan Mine redevelopment	050900000		
	4	3rd Avenue/Madison - 3rd Avenue T-intersection at extended Madison Ave	TBD	Development driven in conjunction with Coughlan Mine redevelopment	046046		
	4	3rd Avenue/Madison - 3rd Avenue T-intersection to Adams Street extension	TBD	Development driven in conjunction with Coughlan Mine redevelopment. This option not supported by Blue Earth County.			
	4	3rd Avenue/Madison - Median at 3rd Avenue	TBD	Development driven in conjunction with Coughlan Mine redevelopment. This option not supported by Blue Earth County.			
	4	Madison Ave to Good Counsel Drive: Reevaluate - with of the street, number of lanes, access to Riverfront, and primary intersection locations	TBD	Consider when infrastructure improvements are needed, land use changes or as opportunities arise with individual business/property owners.			
	5	Construct roundabout at TH 14 South Ramp	\$750,000	Consider when operational and/or safety need is present			
of .	all projects	dependent upon funding availability.					